SMC 20.30.355(C) Decision Criteria	MGP Analysis	Shoreline Analysis
1. The project is consistent with goals and policies of the	As explained in response to Criterion 2, the new	The proposed development is strongly supported by the
Comprehensive Plan. If the project is located within a	Shoreline Place, located in the Comprehensive Plan	City's 2012 Comprehensive Plan. Vision 2029,
subarea plan, then the project shall be consistent with	Mixed Use 1 (MU1) land use designation (CRA), is	developed in 2009, was the City's long-range vision that
the goals and policies of the subarea plan.	centered around vibrant community gathering spaces,	is the basis of the Framework Goals and the goals and
	incorporating multi-family housing, retail, and	policies of the Land Use, Community Design, Housing,
	restaurants where only acres of parking exist. The	Economic Development, and Capital Facilities elements.
	Westminster Way plaza will activate the re-envisioned	In general, these goals and policies support quality
	Westminster Way N. and draw pedestrians into the	development, functionality, walkability, high density,
	Project and its open space system.	business-friendly environment, mixed development with
		more pedestrian/ public spaces and activities, new
	As the City's Parks, Recreation and Open Space Plan	businesses and employers, and economic growth. More
	(PROS Plan) explains, parks, open spaces, and	explicitly, two policies promote a vision, strategy for
	recreational opportunities play a critical role in "who the	mixed uses in Aurora's retail centers and a master
	city is becoming." PROS Plan, Executive Summary,	planned, sustainable, life-style center at Aurora Square.
	page 4. The Conceptual Guide Plan includes a	
	publicly-accessible Open Space System which will	Although, the Community Renewal Area Plan is not a
	create a series of spaces spread throughout the Project	subarea plan it functions like a subarea plan because it is
	varying from green lawns for summer day picnics,	an adopted plan and gives statements that provide vision,
	outdoor movies and concerts in the park, to intimate	goals and policies that become a framework for
	plazas and paseos adjacent to lively restaurants and	implementation (Attachment C).
	retail shops, well-appointed with comfortable seating	
	areas and creative landscape and hearts gave elements.	Comprehensive Plan Framework Goals and Elements
	It will provide pedestrian and bicycle access to and	Goals and Policies (excerpts)
	through Shoreline Place and be connected to the public	
	and private improvements implementing the City's	Framework Goals
	reimagined Westminster Way N. and to the Interurban	FG 9: Promote quality building, functionality, and
	Trail. Figures 2.4 and 4.14 of the PROS Plan illustrate	walkability through good design and development that is
	that there are no neighborhood parks within a 15-	compatible with the surrounding area.
	minute walkshed of Shoreline Place. (The City has	FG 14: Designate specific areas for high density
	established a level of service target for providing	development, especially along major transportation
	neighborhood parks within a 15-minute walk to all city	corridors.
	residents.) The Open Space System will help close this	

# Proposed DA - Att. G - Westminster Way Improvements

<ul><li>gap. One of the many benefits of the open space system is that it will accommodate the Farmers' Market.</li><li>The Project implements Comprehensive Plan Goals and Policies:</li></ul>	FG 15: Create a business- friendly environment that supports small and local businesses, attracts large businesses to serve the community and expand our jobs and tax base, and encourages innovation and creative partnerships.
<ul> <li>Land Use:</li> <li>Goal LU I (encourage development that creates a variety of housing, shopping, entertainment, recreation, gathering spaces, employment, and services that are accessible to neighborhoods).</li> <li>Goal LU VI (encourage pedestrian-scale design).</li> <li>Goal LU VII (plan for commercial areas that serve the community, are attractive, and have long-term economic vitality).</li> <li>Goal LU VII (encourage redevelopment of the Aurora corridor).</li> <li>Goal LU XII (increase access to healthy food by encouraging farmers markets)</li> <li>Policy LU9 (the MU1 designation encourages the development of walkable places with architectural interest that integrate a wide variety of retail and service uses, along with form-based maximum density residential uses).</li> <li>Community Design: <ul> <li>Goal CD I (promote community redevelopment that is aesthetically pleasing, functional, and consistent with the City's vision).</li> </ul> </li> </ul>	Land Use Element Goal LU VI: Encourage pedestrian-scale design in commercial and mixed-use areas. Goal LU VII: Plan for commercial areas that serve the community, are attractive, and have long-term economic vitality. Goal LU VIII: Encourage redevelopment of the Aurora corridor from a commercial strip to distinct enters with variety, activity, and interest. LU9: The Mixed-Use 1 (MU1) designation encourages the development of walkable places with architectural interest that integrate a wide variety of retail, office, and service uses, along with form-based maximum density residential uses. Transition to adjacent single-family neighborhoods may be accomplished through appropriate design solutions. Limited manufacturing uses may be permitted under certain conditions. <u>Community Design Element</u> Goal CD I: Promote community development and redevelopment that is aesthetically pleasing, functional, and consistent with the City's vision. Goal CD III: Expand on the concept that people using places and facilities draw more people. CD1. Encourage building design that creates distinctive places in the community.

<ul> <li>Goal CD II (design streets to create a cohesive image, including continuous pedestrian improvements that connect to the surrounding neighborhoods).</li> <li>Goal CD III (expand on the concept that people using places and facilities draw more people).</li> <li>Policy CD1 (encourage building design that creates distinctive places in the community).</li> <li>Policy CD3 (encourage commercial, mixed-use, and multi-family development to incorporate public amenities, such as public and pedestrian access, pedestrian-oriented building design, mid-block connections, public spaces, activities, and solar access).</li> <li>Policy CD5 (encourage architectural elements that provide protection from the weather).</li> <li>Policy CD18 (preserve, encourage, and enhance open space as a key element of the community's character).</li> <li>Policy CD20 (provide public spaces of various sizes and types throughout the community).</li> </ul>	CD3.Encourage commercial, mixed–use, and multi- family development to incorporate public amenities, such as public and pedestrian access, pedestrian-oriented building design, mid-block connections, public spaces, activities, and solar access. CD21.Design public spaces to provide amenities and facilities such as seating, lighting, landscaping, kiosks, and connections to surrounding uses and activities that contribute to a sense of security. CD24.Encourage building and site design to provide solar access, as well as protection from weather. <u>Housing Element</u> Goal H I: Provide sufficient development capacity to accommodate the 20-year growth forecast and promote other goals, such as creating demand for transit and local businesses through increased residential density along arterials; and improved infrastructure, like sidewalks and stormwater treatment, through redevelopment. <u>Economic Development Element</u> Goal ED I: Maintain and improve the quality of life in the community by: •Increasing employment opportunities and the job base; •Supporting businesses that provide goods and services to local and regional populations; Goal ED II: Promote retail and office activity to diversify sources of revenue and expand the employment
<ul> <li>community).</li> <li>Policy CD21 (design public spaces to provide amenities and facilities such as seating, lighting, landscaping, kiosks, and connections to surrounding uses and activities that contribute to a sense of security).</li> <li>CD24 (encourage building and site design to provide solar access, and as well as protection from weather).</li> </ul>	<ul> <li>Increasing employment opportunities and the job base;</li> <li>Supporting businesses that provide goods and services to local and regional populations;</li> <li>Goal ED II: Promote retail and office activity to diversify sources of revenue and expand the employment base.</li> <li>Goal ED III: Facilitate private sector economic development through partnerships and coordinating funding opportunities.</li> </ul>

# Proposed DA - Att. G - Westminster Way Improvements

<ul> <li>CD27 (where appropriate and feasible, provide lighting, seating, landscaping and other amenities for sidewalks, walkways, and trails).</li> <li>CD30 (provide pedestrian gathering spaces to unify corners of key intersections involving principal arterials).</li> <li>Housing:         <ul> <li>Goal H I (provide sufficient development capacity to accommodate the 20-year growth forecast and promote other goals, such as creating demand for transit and local businesses through increased residential density along arterials; and improved infrastructure, like sidewalks and stormwater treatment, through redevelopment).</li> <li>Policy H3 (encourage infill development on vacant or underutilized sites).</li> </ul> </li> <li>Transportation:         <ul> <li>Goal T III (provide a pedestrian system that is safe, and connects to destinations, accesses transit, and is accessible by all).</li> <li>Policy T22 (prioritize construction of sidewalks, walkways, and trails. Pedestrian facilities should connect to destinations, access transit, and be accessible by all).</li> <li>Policy T49 (expand the city's pedestrian network).</li> </ul> </li> </ul>	Goal ED VI: Support employers and new businesses that create more and better jobs. Goal ED VII: Encourage multi-story buildings for efficient land use. Goal ED VIII: Promote and support vibrant activities and businesses that grow the local economy. ED1: Improve economic vitality by: •Promoting existing businesses; •Recruiting new businesses; •Encouraging increased housing density around commercial districts, especially those served by high capacity rapid transit, to expand customer base; and •Developing design guidelines to enhance commercial areas with pedestrian amenities, and "protect and connect" adjacent residential areas. ED2: Promote non-motorized connections between commercial businesses, services, and residential neighborhoods. ED4: Use incentives and development flexibility to encourage quality development. ED6: Work to reinvigorate economically blighted areas in Shoreline by establishing Community Renewal Areas with associated renewal plans. ED7: Enhance existing neighborhood shopping and community nodes to support increased commercial activity, neighborhood identity, and walkability. ED8: Explore whether creating an "Aurora Neighborhood" as a fifteenth neighborhood in Shoreline would allow the City to better serve citizens, and to capitalize on its infrastructure investment. ED9: Promote land use and urban design that allows for
• Goal ED I (maintain and improve the quality of life in the community by increasing employment opportunities; supporting businesses that provide goods and services to	capitalize on its infrastructure investment. ED9: Promote land use and urban design that allows for smart growth and dense nodes of transit-supportive

	<ul> <li>local and regional populations; complementing community character).</li> <li>Goal ED II (promote retail activity to diversify sources of revenue and expand the employment base).</li> <li>Goal ED VII (encourage multi-story buildings for efficient land use).</li> <li>Goal ED VIII (promote and support vibrant activities and businesses that grow the local economy).</li> <li>Policy ED6 (reinvigorate economically blighted areas in Shoreline by establishing <i>Community Renewal Areas</i> with associated renewal plans).</li> <li>Policy ED7 (enhance existing neighborhood shopping and community notes to support increased commercial activity, neighborhood identity, and walkability).</li> <li>Policy ED12 (revitalize commercial business districts and encourage high-density mixed-use in these areas).</li> <li>Policy ED 14 (encourage a mix of businesses that complement each other and provide variety to the community to create activity and economic momentum).</li> <li>Policy ED 32 (support farmers market).</li> <li>Parks, Recreation &amp; Open Space: <ul> <li>Policy 1.2 (provide a variety of indoor and outdoor gathering places for recreational and cultural activities).</li> <li>Policy 1.3 (plan for, acquire and develop land for new facilities to meet the need of a growing population).</li> </ul> </li> </ul>	commercial activity to promote a self-sustaining local economy. ED12: Revitalize commercial business districts and encourage high-density mixed-use in these areas. ED14: Encourage a mix of businesses that complement each other and provide variety to the community to create activity and economic momentum. ED23: Encourage the redevelopment of key and/or underused parcels through incentives and public/private partnerships. ED27: Develop a vision and strategies for creating dense mixed-use nodes anchored by Aurora's retail centers, including how to complement, support, and connect them with mid-rise residential, office, and destination retail buildings. ED29: Reinvent Aurora Square to help catalyze a master- planned, sustainable lifestyle destination. <u>Capital Facilities Element</u> CF5: Identify, construct, and maintain infrastructure systems and capital facilities needed to promote the full use of the zoning potential in areas zoned for commercial and mixed-use. CF25: Evaluate and establish designated levels of service to meet the needs of existing and anticipated development.
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	<ul> <li>Goal PRII (provide community-based recreation and cultural programs that are diverse and affordable).</li> <li>Goal PR III (meet the parks, recreation and cultural service needs of the community by equitably distributing resources).</li> <li>Policy 3.3 (equitably distribute facilities and program offerings based on identified need).</li> <li>Policy 3.4 (identify unserved and underserved populations with unmet recreation and cultural needs).</li> </ul>	
	<ul> <li>Policy 4.2 (seek partners and planning, enhancement and maintenance of facilities and programs).</li> <li>Policy 4.4 (engage and partner with the business community to create public open space in private development).</li> </ul>	
Ir bi C th ac bo bi cl th el C	a addition to re-channelizing N. 160 <sup>th</sup> Street to provide cycle lanes, as illustrated on <b>page 105 of the</b> <b>onceptual Guide Plan</b> , bicycle circulation through the Project site will be accommodated for dvanced/commuter riders along the northern property bundary and N. 157 <sup>th</sup> Street in a sharrow lane. Other cycle circulation through the site for residents, mildren and recreational bicyclists will be provided arough a wide multipurpose raised sidewalk. These ements of the Project implement the following omprehensive Plan Goals and Policies:	
	• Land Use Goal LU II (establish land use patterns that promote walking, biking and	

using transit to appear and a complete	
aducation employment recreation	
Transportation Coal T II (develop a biovale	
• Transponation Goal T II (develop a bicycle	
system that is connective, safe, and	
encourages dicycling as a viable alternative	
to driving).	
• Transportation Goal T VI (encourage	
alternative modes of transportation to reduce	
the number of automobiles on the road,	
promote a healthy city, and reduce carbon	
omissions).	
• Transportation Policy T6 (support and	
promote opportunities and programs a	
residents have options to travel using	
modes other than single-occupancy	
vehicles).	
• Transportation Policy T 50 (prioritize	
projects that complete the city's bicycle	
networks).	
<ul> <li>Economic Development Policy ED2</li> </ul>	
(promote non-motorized connections	
between commercial businesses, services,	
and residential neighborhoods).	
The Developer will provide a mid-block crossing on N.	
160 <sup>th</sup> Street which implements Transportation Policy T	
23 (design crossings that are appropriately located and	
provide safety and convenience for pedestrians).	
The City has approved a deviation to allow the N. $160^{\text{m}}$	
Street ADA accessible pedestrian facility to be located	
on the Property, allowing the Developer to retain the	
trees adjacent to the N. 160 <sup>th</sup> right-of-way. Retention	

of these trees is consistent with Policy CD 16 (where feasible, preserve significant trees and mature vegetation); and Policy NE 19 (minimize removal of healthy trees).	
<ul> <li>As discussed in response to Criterion 5, the proposed development utilizes a variety of strategies to respond the residentially zoned areas across N. 160<sup>th</sup> Street. These strategies implement the following Housing Goals and Policies:</li> <li>Goal H V (integrate new development with consideration to design and scale the complements existing neighborhoods and provides effective transitions between different uses and intensities).</li> <li>Policy H 23 (assure that site, landscaping, building, and design regulations create effective transitions between different land uses and densities).</li> </ul>	
<ul> <li>On-site stormwater management will implement: <ul> <li>Land Use Policy LU69 (design, locate and construct surface water facilities to promote water quality).</li> <li>Natural Environment Goal NE VI (manage the stormwater system through the preservation of natural system and structural solutions in order to protect water quality).</li> <li>Natural Environment Goal NE VII (continue to require that natural and on-site solutions, such as infiltration and rain gardens be proven infeasible before considering</li> </ul> </li> </ul>	

engineered solutions, such as detention).	
As explained in response to Criterion 3, the Project satisfies the City's concurrency standards and is consistent with Transportation Policy T 44 (adopted Level of Service D).	
As explained in response to Criteria 3 and 4, there is sufficient capacity and infrastructure (roads, sidewalks, bike lanes) to meet the City's adopted level of service standards and sufficient capacity within public services (water, sewer and stormwater) to adequately serve the development proposal in all future phases. Consequently, the Project satisfies Capital Facilities Goal CF II (ensure the capital facilities and public services necessary to support new development is available, concurrent with locally adopted levels of service and in accordance with Washington State Law).	
As detailed in Section 11 of the Development Agreement, an existing City stormwater line is located on the Property adjacent to the Westminster Way N. right-of-way. The Developer will relocate the City stormwater line from the Property to the Westminster Way N. right-of-way in a coordination with the Alexan project. Relocation of the stormwater line is consistent with Utilities Policy U7 (encourage the co-location or joint use of trenches so that utilities may encourage expansion, maintenance, undergrounding, and upgrading of facilities with the least amount of disruption to the community or of service delivery).	

2. The proposed development uses innovative, aesthetic, energy-efficient and environmentally sustainable architecture and site design.	ed in the middle of the last Century, the Aurora uper-block was conceived of an auto-centric velopment that is largely disconnected from the neighborhood context. The new Shoreline Place ed around vibrant community gathering space, ating multi-family housing, retail, and nts where only acres of parking exist. A new of inter-connected open spaces will include a un street or woonerf, public plazas, and park at prioritize the pedestrian realm and connections the new stormwater system will incorporate act development strategies that integrate with the e design to improve storm water quality. and mixed-use development will mitigate the at island while promoting walkability. New s will be at least 60 percent more energy efficient se originally developed on-site.	<ul> <li>The proposed Development Agreement includes departures from some of the Commercial Design Standards (Attachment E, Section 12 Modifications). These departures are typical and approved for most developments in Shoreline. The Alexan Apartments, being built across Westminster Way N, and Paceline at 172 and Aurora Ave N are prime examples.</li> <li>In general, when reviewing departures from these standards, the City decides whether these developments meet the code's stated intent and views the entire design as well as individual, smaller departures. The inclusion of other design standards that will be met make it difficult to discern the minor departures by this Development Agreement.</li> <li>Building design departures to allow an "articulation feature" from every 35 feet to 80 feet and a façade offset from 20 feet to 10 feet deep are relatively minor amongst all the other features and components of the proposed development.</li> <li>Site design departures such as allowing parking lot walkways to be separated from every 200 feet to 265 feet, allowing sidewalks to be reduced from 8 feet to 6 feet wide, and allowing pavers in landscape areas adjacent to parking stalls are</li> </ul>
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grade change of 15 feet from the lower portion of Shoreline Place to the upper portion. Height is determined by average grade per code. See Criterion 5 for how buildings with 80 feet of height in Transition Areas are modified by greater setbacks and therefore meet code standards.
The proposed development will develop a town center with parks, plazas, internal walkways, and public spaces for day and evening activity. Though the development does not propose the highest possible sustainable building practices, the replacement of the Sears buildings and hardscape with the current International Building Code (IBC) energy and Department of Ecology (DOE) surface water codes will be a large step in that direction.
The proposed approach to development is innovative for Shoreline where most development is one building without any physical, visual, or design connection to the adjacent properties. A mixed use, town center with retail, groceries, residences, office (WSDOT), and a nearby college that has connected walkways and public places will physically hold or draw these activities and land uses together. This development will allow people to find a variety of activities and have less of a need to drive around town to connect them. In addition, this site
will be served by high-capacity transit along Aurora Avenue and bicyclists and pedestrians by way of the Interurban Trail. The proposed development is about 1/3 of the entire CRA and located in the center of Shoreline Place. Its precedence, physical template, and the activity

		of this development will likely initiate positive changes on the adjoining properties when those redevelop.
3. There is either sufficient capacity and infrastructure (e.g., roads, sidewalks, bike lanes) that meet the City's adopted level of service standards (as confirmed by the performance of a transportation impact analysis) in the transportation system (motorized and nonmotorized) to safely support the development proposed in all future phases or there will be adequate capacity and infrastructure by the time each phase of development is completed. If capacity or infrastructure must be increased to support the proposed development agreement, then the applicant must identify a plan for funding their proportionate share of the improvements.	<ul> <li>The Developer has provided and the City has reviewed and approved a Transportation Consistency Analysis which provides a trip generation analysis of the Project; documents that, with buildout of the Project, the Alexan apartments, and addition of 200 employees to the WSDOT Headquarters building, new PM peak hour trips will total 264 trips, well below the 808 trips analyzed in the Aurora Square CRA Environmental Impact Statement (EIS) as addended in March, 2019; and proposes mitigation.</li> <li>Transportation improvements, identified in Section 7 of the Development Agreement, include: Westminster Way N. frontage improvements; a proportionate share contribution to the Westminster Way N./155<sup>th</sup> Street N. intersection; channelization improvements to N. 160<sup>th</sup> Street to provide bike lanes; a mid-block pedestrian crossing on N. 160<sup>th</sup> Street; proportionate share contributions to the Greenwood Avenue N./NW Innis Arden Way and Greenwood Avenue N./N 160<sup>th</sup> Street intersections; and a proportionate share contribution to the Greenwood Avenue N./N.165<sup>th</sup> Street intersection.</li> <li>As explained in Section 20 of the Development Agreement that development of up to 1,358 residential units and 75,610 square feet of commercial space through the year 2039 passes the concurrency test.</li> </ul>	<ul> <li>The development proposal includes a thorough transportation consistency analysis which demonstrates compliance with Shoreline Municipal Code 20.60.140 Adequate Streets and consistency with environmental analysis performed as part of the City-led Community Renewal Area Plan. The development demonstrated compliance in the following ways:</li> <li>The total maximum net new proposed peak hour trips generated by the project is 160. Project trips, in addition to pipeline trips for active projects within the CRA such as the Alexan, remain well under the EIS studied threshold of 808 trips.</li> <li>The proposal build-out year extends to 2039. Current City analysis only extends to 2030. As such, additional analysis was performed to determine the net difference in trips that would be added to the various CRA study intersections in the 2039 out year in comparison to what was studied by the City. The results showed very little difference in comparison to what was studied under Shoreline's environmental analysis; as such no change to the traffic Level of Service outcomes would be anticipated. (Attachment G Exhibit B)</li> <li>The project will contribute proportional share mitigation funding to 2 locations (See</li> </ul>

	Attachment G Exhibit B). shown to be failing the City's level of Service standards in future years, as previously identified by Shoreline Community College analysis.
	• The project will pay Transportation Impact Fees for other citywide impacts, in accordance with the code applicable at the time of permit. This accounts for permits both now, and in out years past 2030, as the City's concurrency standards and associated growth projects may evolve with updated Transportation Master Plan analysis.
	• The project will include a public bike connection through the site for advanced/commuter riders along the northern property boundary controlled by Shoreline Place diagonally through the project in a "sharrow" lane, while other bicycle circulation through the site for residents, children, and recreational bicyclists would be provided through a wide multipurpose raised sidewalk. This bicycle facility will serve as an important connection between the Shoreline Community College and neighborhoods to the west, and the Interurban Trail.
	• The development proposal improves transportation facilities consistent with the CRA plan and slated planned improvement projects in <b>Attachment G Exhibit B.</b>
	• Off-site Transportation Improvements are detailed in Attachment E, Section 7.

4. There is either sufficient capacity within public services such as water, sewer and stormwater to adequately serve the development proposal in all future phases, or there will be adequate capacity available by the time each phase of development is completed. If capacity must be increased to support the proposed development agreement, then the applicant must identify a plan for funding their proportionate share of the improvements.	<ul> <li><u>Utilities Generally</u>: Section 10 of the Development Agreement requires that the Developer construct the requisite water, sanitary sewer, and stormwater facilities (the "Utilities") onsite and pay any connection fees and impact fees due for utility facilities located offsite as part of the buildout of the Project. No off-site utility improvements within the City's control are required. Developer is responsible for the costs associated with alteration or extension of on-site utility infrastructure necessary to connect to the City's infrastructure.</li> <li><u>Water</u>: As explained in the CRA EIS, the City of Seattle was provided with a description of the growth planned for the CRA and indicated that the water system has capacity for this growth. Section 23 of the Development Agreement requires that the Developer provide the City with a Water Availability Certificate with all building permit applications requiring the provision of potable water or fire flow.</li> <li><u>Sewer</u>: As explained in Section 24 of the Development Agreement, the Ronald Wastewater District City has analyzed its existing and future sanitary sever capacity and infrastructure. Based on its review for the next 25 years, the District has acknowledged that there is sufficient local sanitary sever capacity and infrastructure in place or planned to serve the Project and that Developer may construct on- site capacity and connect to the City's sanitary sewer system to serve the Project subject to review and approval of a Developer Extension Agreement. The</li> </ul>	<ul> <li>The proposed Development Agreement adequately addresses the capacity for services for water, sewer and stormwater based on the Conceptual Guide Plan. The following is summary of each utility:</li> <li>Water: MGP has had preliminary discussions with Seattle Public Utilities who have indicated there is or can be adequate water supply.</li> <li>Sewer: Ronald Wastewater District is reviewing their capacity analysis. RWD has indicated there are no significant issues that cannot be addressed through the development process.</li> <li>Storm: The City is in discussion with MGP regarding the relocation of an existing stormwater pipe (does not serve the Sears site) from their property into the Right of Way of Westminster Way because it will limit the placement of proposed buildings on that part of the site and therefore have ramifications on the remainder of the site and urban design. The Agreement will address the responsibility and timing of the stormwater pipe relocation. MGP will be responsible to manage stormwater on their project site in accordance with the stormwater regulations in place at the time of each phase of development.</li> <li>In general, MGP will be responsible to coordinate directly with the utility providers to ensure</li> </ul>

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	Developer has applied to Ronald Wastewater District for a Developer Extension Agreement. <u>Stormwater</u> : It is anticipated that stormwater will be managed on-site and that no off-site improvements are necessary. <i>See also</i> , <b>Section 22 of the Development</b> <b>Agreement</b> which requires that all stormwater facilities meet current City, state, and federal regulations in effect at the time of application for the permit triggering the need for stormwater facilities. Such stormwater facilities will provide a substantial improvement over existing conditions. The City has acknowledged that it is not aware of capacity constraints in the natural conveyance system in the event that Developer's stormwater will discharge off- site to the natural environment in addition to the use of onsite detention/vaults in accordance with applicable local and state requirements.	capacity is available at each phase of the development.
5. The development agreement proposal contains architectural design (including but not limited to building setbacks, insets, facade breaks, roofline variations) and site design standards, landscaping, provisions for open space and/or recreation areas, retention of significant trees, parking/traffic management, multimodal transportation improvements, and other features that minimize conflicts and create transitions between the proposal site and adjacent property zoned R-4, R-6, R-8 or MUR-35'.	The proposed development utilizes a variety of strategies to respond the residentially zoned areas across N. 160 <sup>th</sup> Street. Commercial uses and active open spaces are oriented towards the eastern portion of the site, transitioning to solely residential uses closer to the existing residential zone. Significant building setbacks are provided along N.160 <sup>th</sup> Street to retain the existing densely vegetated frontage and provide for an internal pedestrian pathway adjacent to the street. In addition to code required building modulation, the upper floor façades adjacent to residential zones will use material changes or setbacks to reduce the perceived height and scale.	The portion of this criterion related to minimizing conflicts and creating transitions between the project and R-4, R-6, R-8 or MUR-35', only applies to the R-6 zoned neighborhood directly across from the proposal on the north side of N 160 <sup>th</sup> Street. That R-6 area fronts approximately 270 feet directly across from the Sears site and includes three single-family residences. The proposed Development Agreement shows proposed buildings on the south side of N. 160 <sup>th</sup> Street directly across from these R-6 residences. The proposed buildings will meet most of the City's Commercial Design Standards (see Criterion 2 above).

		In addition, the City has Transition Area Standards that require the proposed buildings to be setback and the upper stories stepped-back to minimize the apparent building size toward the R-6 homes. The proposed buildings will meet this standard by increasing the setback at the building base from 10 feet to 60 feet. This increased setback allows the proposed building height to be 80 feet in height and to meet the Transition Area standards.
6. The project is consistent with the standards of the critical areas regulations, Chapter 20.80 SMC, Critical Areas, or Shoreline Master Program, SMC Title 20, Division II, and applicable permits/approvals are obtained.	<ul> <li><u>Critical areas</u>: The Project is located south of Boeing Creek, which is piped within N. 160<sup>th</sup> Street near the northeast property line. SMC Table 20.80.280(1) requires a 10' buffer for a piped stream. Work will occur within the buffer for driveway and right-of-way improvements as permitted by SMC 20.80.274.C.4.</li> <li><u>Shoreline Master Program</u>: The SMP is not applicable. The Property is not within the shoreline jurisdiction.</li> </ul>	There are no Wetland, Floodplain, Fish and Wildlife Habitat (stream corridors), Flood Hazard Areas, or Aquifer Recharge critical areas on site. "Piped Streams" are included in the Critical Area Code; however, they only exist along the proposed N 160 <sup>th</sup> Street improvements. The proposed relocation of the stormwater pipe in Westminster is not designated as a "Piped Stream". There are some isolated, man-made, moderately hazardous and minorly high hazardous steep slope indicated on the Sears site. These slopes were originally created to level parking and building pad areas. The conceptual development plans are not specific enough to discern whether this is a conflict. However, the proposed Development Agreement is not requesting a departure from the critical areas code. The Sears site is not within the Shoreline Master Program area.

## Attachment G

## Vision 2029 and Comprehensive Plan Policies (Excerpts)

#### Vision 2029 Statement

Shoreline in 2029 is a thriving, friendly city where people of all ages, cultures, and economic backgrounds love to live, work, play and, most of all, call home. Whether you are a first-time visitor or long-term resident, you enjoy spending time here. There always seems to be plenty to do in Shoreline -- going to a concert in a park, exploring a Puget Sound beach or dense forest, walking or biking miles of trails and sidewalks throughout the city, shopping at local businesses or the farmer's market, meeting friends for a movie and meal, attending a street festival, or simply enjoying time with your family in one of the city's many unique neighborhoods....

Shoreline is culturally and economically diverse, and draws on that variety as a source of social and economic strength...

Schools, parks, libraries, restaurants, local shops and services, transit stops, and indoor and outdoor community gathering places are all easily accessible, attractive and well maintained...

Gathering places - like parks, plazas, cafes and wine bars - provide opportunities for neighbors to meet, mingle and swap the latest news of the day...

If you live nearby, sidewalks connect these hubs of activity to the surrounding neighborhood, bringing a car-free lifestyle within reach for many...

Aurora Avenue is Shoreline's grand boulevard. It is a thriving corridor, with a variety of shops, businesses, eateries and entertainment, and includes clusters of some mid-rise buildings, well-designed and planned to transition to adjacent residential neighborhoods gracefully. Shoreline is recognized as a business-friendly city. Most services are available within the city, and there are many small businesses along Aurora, as well as larger employers that attract workers from throughout the region. Here and elsewhere, many Shoreline residents are able to find family-wage jobs within the City. Housing in many of the mixed-use buildings along the boulevard is occupied by singles, couples, families, and seniors. Structures have been designed in ways that transition both visually and physically to reinforce the character of adjacent residential neighborhoods. The improvements put in place in the early decades of the 21st century have made Aurora an attractive and energetic district that serves both local residents and people from nearby Seattle, as well as other communities in King and Snohomish counties. As a major transportation corridor, there is frequent regional rapid transit throughout the day and evening. Sidewalks provide easy access for walking to transit stops, businesses, and connections to adjacent neighborhoods. Aurora has become a green boulevard, with

## Proposed DA - Att. A/Exhibit A - Vision 2019 & Comp. Plan Policies Excerpts

mature trees and landscaping, public plazas, and green spaces. These spaces serve as gathering places for neighborhood and citywide events throughout the year. It has stateof-the-art stormwater treatment and other sustainable features along its entire length. As you walk down Aurora you experience a colorful mix of bustling hubs – with well-designed buildings, shops and offices – big and small – inviting restaurants, and people enjoying their balconies and patios....

2012 Comprehensive Plan (Excerpts Below).

FG 9: Promote quality building, functionality, and walkability through good design and development that is compatible with the surrounding area.

FG 14: Designate specific areas for high density development, especially along major transportation corridors.

FG 15: Create a business- friendly environment that supports small and local businesses, attracts large businesses to serve the community and expand our jobs and tax base, and encourages innovation and creative partnerships.

Land Use Element

Goal LU VI: Encourage pedestrian-scale design in commercial and mixed-use areas.

Goal LU VII: Plan for commercial areas that serve the community, are attractive, and have long-term economic vitality.

Goal LU VIII: Encourage redevelopment of the Aurora corridor from a commercial strip to distinct enters with variety, activity, and interest.

LU9: The Mixed-Use 1 (MU1) designation encourages the development of walkable places with architectural interest that integrate a wide variety of retail, office, and service uses, along with form-based maximum density residential uses. Transition to adjacent single-family neighborhoods may be accomplished through appropriate design solutions. Limited manufacturing uses may be permitted under certain conditions.

#### Community Design

Goal CD I: Promote community development and redevelopment that is aesthetically pleasing, functional, and consistent with the City's vision.

Goal CD III: Expand on the concept that people using places and facilities draw more people.

CD1. Encourage building design that creates distinctive places in the community.

CD3.Encourage commercial, mixed—use, and multi-family development to incorporate public amenities, such as public and pedestrian access, pedestrian-oriented building design, mid-block connections, public spaces, activities, and solar access.

## Proposed DA - Att. A/Exhibit A - Vision 2019 & Comp. Plan Policies Excerpts

CD21.Design public spaces to provide amenities and facilities such as seating, lighting, landscaping, kiosks, and connections to surrounding uses and activities that contribute to a sense of security.

CD24.Encourage building and site design to provide solar access, as well as protection from weather.

Housing

Goal H I: Provide sufficient development capacity to accommodate the 20-year growth forecast and promote other goals, such as creating demand for transit and local businesses through increased residential density along arterials; and improved infrastructure, like sidewalks and stormwater treatment, through redevelopment.

Economic Development

Goal ED I: Maintain and improve the quality of life in the community by:

•Increasing employment opportunities and the job base;

•Supporting businesses that provide goods and services to local and regional populations;

Goal ED II: Promote retail and office activity to diversify sources of revenue, and expand the employment base.

Goal ED III: Facilitate private sector economic development through partnerships and coordinating funding opportunities.

Goal ED VI: Support employers and new businesses that create more and better jobs.

Goal ED VII: Encourage multi-story buildings for efficient land use.

Goal ED VIII: Promote and support vibrant activities and businesses that grow the local economy.

ED1: Improve economic vitality by:

•Promoting existing businesses;

•Recruiting new businesses;

•Encouraging increased housing density around commercial districts, especially those served by high capacity rapid transit, to expand customer base; and

•Developing design guidelines to enhance commercial areas with pedestrian amenities, and "protect and connect" adjacent residential areas.

ED2: Promote non-motorized connections between commercial businesses, services, and residential neighborhoods.

ED4: Use incentives and development flexibility to encourage quality development.

## Proposed DA - Att. A/Exhibit A - Vision 2019 & Comp. Plan Policies Excerpts

ED6: Work to reinvigorate economically blighted areas in Shoreline by establishing Community Renewal Areas with associated renewal plans.

ED7: Enhance existing neighborhood shopping and community nodes to support increased commercial activity, neighborhood identity, and walkability.

ED8: Explore whether creating an "Aurora Neighborhood" as a fifteenth neighborhood in Shoreline would allow the City to better serve citizens, and to capitalize on its infrastructure investment.

ED9: Promote land use and urban design that allows for smart growth and dense nodes of transit-supportive commercial activity to promote a self-sustaining local economy.

ED12: Revitalize commercial business districts, and encourage high-density mixed-use in these areas.

ED14: Encourage a mix of businesses that complement each other, and provide variety to the community to create activity and economic momentum.

ED23: Encourage the redevelopment of key and/or underused parcels through incentives and public/private partnerships.

ED27: Develop a vision and strategies for creating dense mixed-use nodes anchored by Aurora's retail centers, including how to complement, support, and connect them with mid-rise residential, office, and destination retail buildings.

ED29: Reinvent Aurora Square to help catalyze a master-planned, sustainable lifestyle destination.

#### Capital Facilities

CF5: Identify, construct, and maintain infrastructure systems and capital facilities needed to promote the full use of the zoning potential in areas zoned for commercial and mixed-use.

*CF25:* Evaluate and establish designated levels of service to meet the needs of existing and anticipated development.

Transportation Engineering NorthWest

# **MEMORANDUM**

DATE:	April 8, 2019
TO:	Kendra Dedinsky, PE, City of Shoreline Traffic Engineer Jamas Gwillam, Merlone Geier Partners
FROM:	Michael J. Read, PE, Principal, TENW
SUBJECT:	Shoreline Place - Transportation Consistency/Traffic Impact Analysis <i>Expanded Study</i>

This memorandum documents both a transportation consistency analysis of specific transportation evaluations and land use assumptions documented in the *Aurora Square Community Renewal Area (CRA)* and project-level EIS analysis, as well as a traffic impact analysis to ensure functionality of existing and proposed site access roadways and operations of intersections consistent with the City of Shoreline Concurrency requirements based upon the proposed *Shoreline Place* redevelopment project. As one of the first redevelopment projects within the Aurora Square CRA, this consistency and traffic impact analysis provides the following transportation-related items:

- A comparative trip generation analysis of the *Shoreline Place* project in contrast to existing commercial retail uses within the Sears property using trip generation rates published by ITE in the *Trip Generation Manual*, 10<sup>th</sup>, Edition, 2017. A comparative analysis with the EIS assumptions and conclusions document the remaining trip entitlements within the overall Aurora Square CRA.
- A review of traffic operations and vehicle queueing at existing or proposed future driveways (two existing on N 160<sup>th</sup> Street and two onto Westminster Way) as well as several off site intersections with buildout of *Shoreline Place* to ensure City Transportation Concurrency standards are met. Traffic forecasts consider growth rates applied in the *Aurora Square Planned Action EIS*, buildout of the adjacent Alexan Apartment project, and employment growth expected at the WSDOT Headquarters building.
- A review of existing/proposed truck access driveways to ensure no impacts to adjacent existing retail owners/tenants within the Aurora Square CRA and an identification of truck access driveways and routing to ensure goods mobility and freight deliveries.
- > An overview of a designated bikeway through the *Shoreline Place* project site.
- An overview of transportation improvements necessitated as a result of frontage along public rights-of-way and contributions towards specific projects determined as Priority Renewal projects within the Aurora Square CRA.

# Project Trip Generation

Using the latest edition of the *Trip Generation Manual, 10<sup>th</sup> Edition,* 2017, TENW prepared the estimated vehicle trip generation as a result of the *Shoreline Place* project within the Aurora Square CRA. A site vicinity map with study intersections is provided in **Figure 1**, while a conceptual site plan is provided in **Attachment A**. For trip generation/traffic analysis purposes, a total of 1,358

Shoreline Place Transportation Consistency Analysis - *Expanded Study FINAL* 



residential apartments with 59,160 square-feet of new retail and 13,000 square-feet of sit-down quality restaurant were assumed. An existing bank with drive-thru within the property boundary (3,450 square feet) would remain.

The primary retail center that would be removed as part of the *Shoreline Place* project is a former Sears Retail complex. For the purposes of the transportation analysis, the series of buildings comprised approximately 143,753 square-feet of net retail space, 72,193 square-feet of net warehouse/storage (appliance distribution center), 20,000 square-feet of office (administrative/callcenter), and 50,042 square-feet in auto center/tire store services. It should be noted that approximately 286,000 square feet of existing commercial building space was assumed to be removed as part of the *Shoreline Place* project for traffic analysis purposes (with a total floor area of up to 333,600 square-feet determined as of March 2019).

As shown in **Table 1**, a total of approximately 505 new a.m. peak hour (142 entering and 363 exiting), and 651 new p.m. peak hour vehicular trips (392 entering and 259 exiting) would be generated at full buildout of *Shoreline Place*. It should be noted that these net vehicle trips do not account for removal of the Sears retail complex or pass-by trips. Detailed trip generation tables are provided in **Attachment B**, and where appropriate, fitted curve equations were applied. Consistent with the CRA EIS assumptions, NCHRP 684 methodology was applied to estimate internal trips that would remain within the development.

Table 1: Shoreline Plac	e Net Trip C	<b>Generation S</b>	ummary
Time Period	In	Out	Total
Standard ITE Rates w	vith Pass-By/li	nternal Adjustr	nents
Weekday AM Peak Hour	142	363	505
Weekday PM Peak Hour	392	259	651

Source: Trip Generation Manual, 10<sup>th</sup> Edition, ITE, 2017.

# Comparative Trip Generation Analysis with the CRA EIS

The *Aurora Square Planned Action ElS* modeled three redevelopment scenarios, with redevelopment that would include remove and replace approximately 433,000 square-feet of existing retail uses and construct up to 1,000 new apartment units, 250,000 square-feet of additional retail and 250,000 square-feet of new office uses under the highest approved land use scenario (Alternative 3).

As summarized in **Table 2**, the relative increase in vehicle trip generation contemplated within the *Aurora Square Planned Action ElS* would range from zero (0) under the No Action Alternative to approximately 808 new p.m. peak hour trips under Alternative 3 (City adopted Alternative 3 as referenced in the <u>Addendum</u> to the *Aurora Square Planned Action Draft Environmental Impact Statement* (December 12, 2014) and *Final Environmental Impact Statement* (July 24, 2015), issued on March 8, 2019.

Under the *Shoreline Place* redevelopment scenario (accounting for removal of existing Sears retail complex and internal/pass-by trips), a net increase of 99 new p.m. peak hour trips (132 new entering trips and 33 fewer existing trips) would be generated in comparison to existing land uses. When considering Shoreline Place, the adjacent Alexan Apartment complex, and WSDOT employment growth, the net change is estimated at 264 new trips in total, well below

the EIS threshold of 808 new p.m. peak hour trips under a cumulative comparison. A detailed summary of land use and vehicle trip generation assumptions of the EIS trip thresholds is provided in **Attachment C**. As such, the proposed Shoreline Place project is consistent with the traffic analysis and land use assumptions of the *Aurora Square Planned Action EIS*.

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Scenario	In	Out	Total	Net Change
No Action	453	594	1,047	0
Alternative 2	633	812	1,445	+398
Alternative 3	817	1,038	1,855	+808
Shoreline Place (only)	585	561	1,146	+99
Shoreline Place + Alexan Apts (+85)+ WSDOT Growth (+80)	+87=672	+78=629	+165=1,311	+264

#### Table 2: EIS PM Trip Threshold Comparisons

Source: Aurora Square CRA EIS and the Trip Generation Manual, 10<sup>th</sup> Edition, ITE, 2017.

## Traffic Impact Analysis

To ensure compliance with the City's Transportation Concurrency standards and to evaluate consistency with the traffic operational results of the *Aurora Square Planned Action ElS* transportation analysis, intersection level of service (LOS) analyses were conducted at all site driveways and additional off-site intersections northwest of the Shoreline Place project per the direction of the City Traffic Engineer. Intersection level of service analysis reviewed future baseline conditions under a 20-year buildout horizon year (2039) and with new project traffic generated at full buildout of the *Shoreline Place* project.

## 2039 Baseline Traffic Forecasts

To evaluate project traffic operations at full buildout of *Shoreline Place*, review of the traffic projections within the *Aurora Square Planned Action ElS* was conducted by TENW. As provided in **Attachment D**, the average annual growth rate between 2013 and 2030 determined in the ElS was 1.2 percent per year of total entering volumes at study intersections (see **Figure 1**) that immediately serve the site. As such, to prepare 2039 forecasts under the buildout horizon year for Shoreline Place, this average annual growth rate was applied to all study intersections for a 21-year period using an extrapolation method of 21-years growth on traffic counts collected in 2018.

In addition to these background traffic growth rates, known pipeline traffic associated with the Alexan Apartment complex and additional employment expected at the adjacent WSDOT Headquarters building (200 new employees from the Department of Ecology) within the Aurora Square CRA area were added to these forecasts. The resultant cumulative total growth factor applied to study intersections and site driveways ranged from approximately 25 percent to 33 percent between 2018 and 2039.

To evaluate project traffic impacts, the total net increase in vehicle trip generation from the Shoreline Place project were added to the baseline 2039 traffic forecasts with additional adjustments for passby trips associated with proposed retail land uses. As the 2018 traffic counts did not account for any significant vehicle trip generation associated with the Sears retail complex, no adjustments to existing counts were made as a conservative approach. Adjustments were made however, to account for removal of the off-site student parking/shuttle service that currently occurs on-site to support the Shoreline Community College. **Attachment D** also provides existing 2018 traffic counts and the resultant p.m. peak hour traffic forecasts for 2039 under future baseline and with project conditions.

## Consistency

As part of the consistency review, a comparative analysis of detailed EIS forecasts at the intersection-level was completed with additional factoring to 2039 to ensure growth projections used in evaluation of traffic impacts associated with Shoreline Place were consistent with assumptions within the CRA EIS. As provided in **Attachment E**, totaling entering volumes at each study intersection within the CRA is documented and factored to 2039 under the No Action Scenario. Net increases in vehicle trips associated with known buildout within the CRA including the Alexan Apartment complex, new WSDOT trips, and Shoreline Place are identified. The resultant intersection-level total entering volumes in 2039 "with known CRA buildout" were then compared with the adopted Alternative 3 forecasts (representing a 2030 horizon year), and on an aggregate basis totaling entering traffic at all study intersections is approximately 3.7 percent lower than those levels evaluated in the CRA EIS.

At an intersection level, those study intersections that were slightly higher than EIS forecasts were forecasted at no more than 2.7 percent or less, and at those intersections within Shoreline's jurisdiction, were projected to operate at LOS D or better. At those intersections where EIS forecasts are higher using these methods, projected level of service is better than that documented in the CRA EIS. This is primarily due to the fact that the overall net increase in traffic generated by uses within the CRA is significantly lower than EIS assumptions.

#### Project Trip Distribution

To estimate vehicle trip distribution, existing traffic patterns at regional intersections and site access driveways as well as the distribution of land uses and proposed parking facilities on-site were all considered. As provided in **Figure 2** portrays off-site trip distribution patterns as well as general site driveway trip distribution patterns. Detailed trip assignments of turning movements and directional flows are provided in **Attachment D** as part of 2039 Traffic Forecasts.

#### Intersection Level of Service Analysis

LOS refers to the degree of congestion on a roadway or intersection. It is a measure of vehicle operating speed, travel time, travel delays, and driving comfort. A letter scale from A to F generally describes LOS. At signalized intersections, LOS A represents free-flow conditions-motorists experience little or no delays, and LOS F represents forced-flow conditions-motorists experience an average delay in excess of 80 seconds per vehicle. The LOS reported for signalized intersections represents the average control delay per vehicle entering the intersection. The LOS reported at stop-controlled intersections is also based on the average control delay (sec/veh) and is reported for each movement. Therefore, the reported LOS at unsignalized intersections does not represent a measure of the overall operations of the intersection.

LOS calculations for both signalized and stop-controlled intersections were calculated using the methodologies and procedures outlined in the 2010 *Highway Capacity Manual (HCM)*, Special

#### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis

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Transportation Consistency Analysis – Expanded Study FINAL

Report 209, Transportation Research Board (TRB). **Table 3** outlines the LOS criteria for signalized and unsignalized intersections based on these methodologies.

	Signalized Intersection	Unsignalized Intersection
Level of Service	Average Delay Range (sec)	Delay Range (sec)
А	≤ 10	≤ 10
В	> 10 to ≤ 20	> 10 to ≤ 15
С	> 20 to ≤ 35	> 15 to ≤ 25
D	> 35 to ≤ 55	> 25 to ≤ 35
E	> 55 to ≤ 80	> 35 to ≤ 50
 F	> 80	> 50

#### Table 3: Level of Service Criteria for Intersections

Source: "Highway Capacity Manual", Special Report 209, Transportation Research Board, 2010.

To evaluate consistency with the Aurora Square CRA EIS, intersection LOS were calculated using HCM 2010 procedures with the *Synchro 8* software program at all primary access intersections and off-site vicinity intersections northwest of the site that would serve the site. Baseline and With Project forecasts in 2039 during the p.m. peak hour are summarized in **Table 4**. In December 2018, the City of Shoreline and Shoreline Community College (SCC) executed a Transportation Mitigation Agreement in support of the College's buildout of student housing and Master Plan. As part of this effort, detailed modeling of transportation mitigation alternatives was completed by SCC and approved by the City of Shoreline. As the net increase in new trips generated by Shoreline Place is negligible at Study Intersections 1, 2, and 3, and as the Transportation Mitigation Agreement, the traffic analysis and long range improvements at these off-site intersections are documented in *Shoreline Community College Transportation Technical Report*, October 2018, Transpo Group, Inc.

Figure 3 overviews p.m. peak hour traffic forecasts in 2039 at study intersections 4 through 9, also denoting the net increase in p.m. peak hour vehicle trips that would be generated by the project. As shown, all intersections or critical stopped controlled movements are forecast to operate at LOS D or better with or without Shoreline Place in 2039. Detailed level of service summary sheets are also included in Attachment F. As reported in the *Shoreline Community College Transportation Technical Report*, October 2018, Transpo Group, Inc., Study Intersections 1, 2, and 3 would operate at LOS D or better with planned transportation mitigation by SCC and the City of Shoreline (traffic forecasts and level of service results are provided in Attachment G).

#### Queuing Analysis at Site Driveways

Using HCM 2010 procedures, **Table 4** also includes 95<sup>th</sup>-percentile vehicle queuing estimates of critical entering/existing lanes or flows at site driveways or relevant study intersections. As shown, all exiting site driveways are estimated to experience vehicle queues of no more than 2 vehicles at stop controlled approaches and up to 6 vehicles (less than 150 feet) at the signalized approach of Westminster Way and N 155<sup>th</sup> Street. Supporting total entering and exiting traffic associated with Shoreline Place development are also summarized in **Table 4** for disclosure of trip assignments that were used in the evaluation of project-level traffic impacts.

Shoreline Place Transportation Consistency Analysis – *Expanded Study FINAL* 



		Analysis	PM	Peak	With Project Vehicle
#OI	Intersection	Method	2039 Baseline	2039 With Project	Queves Enter/Exit (95th Percentile)
4	Dayton Ave/N 160th St	нсм	В	B	WB LT - 50 feet WB TH - 150 feet
5	Sears West Dr/N 160th St	НСМ	C	U	NB LT - 25 feet NB RT - 25 feet
9	ROIC Dr/N 160th St	HCM	U	U	NB LT/RT - 50 feet
7	Westminster Wy/Pier 1 Dr	HCM	A	В	EB TH/RT - 25 feet
					EB TH/RT - 125 feet NB LT - 250 feet
8	Westminster Wy/N 155th St	HCM	В	В	EB TH - 150 feet
6	New SP NW Dr/N 160th St	HCM	-	В	NB LT/RT - 25 feet
As noted ir Technical I	1 the study document, Study Intersections 1 through 3 Report, Transpo Group, October 2018.	are not reported i	n this analysis, but by refe	srence are reported in the St	noreline Community College Transportation

Project Trip Generation (Gross Trips)	Enter	Exit	Total
AM Peak Hour	166	388	554
PM Peak Hour	477	348	825
Project Trip Generation (Net New Trips)	Enter	Exit	Total
AM Peak Hour	142	363	505
PM Pack Hour	307	750	451

PM Peak Hour	477	348	825	
Project Irip Generation (Net New Irips)	Enter	Exit	Total	Net New Trips Removes Internal Trips
AM Peak Hour	142	363	505	between Uses within Shoreline Place
PM Peak Hour	392	259	651	and within the Aurora Square CRA
				and Removes Pass-by Trips. These
Pass-By Trips Reassigned at Driveways	Enter	Exit	Total	figures do not account for removal of
AM Peak Hour	5	5	10	ine existing sears retail complex.
PM Peak Hour	24	25	49	

Shoreline Place Transportation Consistency Analysis – *Expanded Study FINAL* 

## Truck Routes

Large wheel-based vehicles (semi-truck/trailer combinations) currently make regular deliveries to four separate loading docks throughout the retail core of the existing Aurora Square CRA properties. With removal of the Sears retail complex, deliveries by these large vehicles are not expected as a result of the small retail shops proposed within the Shoreline Place development. However, continued truck deliveries using WB-62 combinations would require access to Marshal's, Central Market (2 separate locations), and the Salvation Army Family Store. Figure 4 identifies these four loading dock locations and outlines existing/proposed truck entry/exit points around the Shoreline Place development. To continue truck accessibility, the planned relocated WSDOT access onto N 160<sup>th</sup> Street within the northwest quadrant of the site would provide a majority of this accessibility, while a proposed new driveway to serve truck "entering vehicles only" at the Marshall's loading dock from N 160<sup>th</sup> Street immediately east of the primary Shoreline Place driveway is proposed.

## Interior Bike Routes

Given the mixed use nature of the *Shoreline Place* project and the overall Aurora Square CRA community priorities and redevelopment goals, bike circulation "through the site" would be accommodated for advanced/commuter riders along the northern property boundary controlled by *Shoreline Place* diagonally through the project in a sharrow lane, while other bicycle circulation through the site for residents, children, and recreational bicyclists would be provided through a wide multipurpose raised sidewalk. **Figure 5** overviews these two bicycle routes through the site.

# Transportation Mitigation

With establishment of the *Aurora Square CRA* and Renewal Plan, the City master planning identified a number of projects that the City of Shoreline can accomplish in partnership with redevelopment. The transportation improvements identified through the Planned Action EIS process were prioritized to support economic renewal of the Aurora Square CRA area. Of the eleven transportation improvements identified in Table A-1. Renewal Priority of Aurora Square CRA Transportation Improvements, six were a high priority, including:

- Project 1. Rechannelization of N 160th St from Dayton Avenue to Aurora Avenue to a 3lane section with bike lanes and transition back to signalized approaches.
- Project 2. Midblock pedestrian crossing of N 160<sup>th</sup> Street with a refuge to provide for safe pedestrian crossings given the distance between Dayton Avenue and Aurora Avenue N.
- Project 3. Provide a new bicycle connection between the Interurban Trail and N 160<sup>th</sup> Street along Westminster Way.
- Project 4. Redevelop Westminster Way N from N 155<sup>th</sup> Street to N 160<sup>th</sup> Street to provide a more pedestrian and bicycle friendly section with street parking.
- > Project 5. Construct N 157<sup>th</sup> Street from Westminster Way N to Aurora Ave N.
- Project 6. Reconstruct the signalized intersection at N 155th St and Westminster Way N in conjunction with the Westminster Way N project to increase safety for pedestrians and bicycles. Includes improvements to the section of N 155th St between Westminster Way N and Aurora Ave.

Shoreline Place Transportation Consistency Analysis – *Expanded Study FINAL* 



Transportation Consistency Analysis – Expanded Study FINAL



As part of the project, through construction of frontage improvements, contributions towards the City's Westminster Way N and Westminster Way N/N  $155^{\text{th}}$  Street intersection project, and construction of Project 2, Shoreline Place would be a major contributor to these high priority transportation improvements:

Project 1. Shoreline Place would be responsible for rechannelization of N 160<sup>th</sup> Street for approximately 1,200-lineal feet of frontage (with transitions beyond) to provide 3 travel lanes and bike lanes on both sides of the street (see **Attachment H** of N 160<sup>th</sup> Street roadway section) with appropriate transition back to Linden Avenue N and Dayton Avenue N. This channelization improvement replaces the previously identified "cycle track" project along N 160<sup>th</sup> Street.

Project 2. Shoreline Place would construct this mid-block pedestrian crossing treatment (Rectangular Rapid Flash Beacon or RRFB) on the east leg of the N 160<sup>th</sup> Street and Fremont Place N.

Project 3. Completed with contributions by *Shoreline Place* towards the City's Westminster Way N and Westminster Way N/N 155<sup>th</sup> Street intersection project.

Project 4. Completed with contributions by *Shoreline Place* towards the City's Westminster Way N and Westminster Way N/N 155<sup>th</sup> Street intersection project.

Project 6. Completed with contributions by *Shoreline Place* towards the City's Westminster Way N and Westminster Way N/N 155<sup>th</sup> Street intersection project.

To mitigate *Shoreline Place* impacts at off-site study intersections beyond the CRA study area, as part of the City's executed Transportation Mitigation Agreement with SCC, a proportional share basis of 3 new p.m. peak hour trips is estimated for *Shoreline Place* towards transportation improvements required by the SCC campus at the intersections of Dayton Avenue N/Carlyle Hill Road (out of a total of 21 p.m. peak hour by SCC), and 3 new p.m. peak hour trips at the adjacent intersections of Greenwood Avenue N/Innis Arden, and Greenwood Avenue N/N 160<sup>th</sup> Street (out of a total of 58 p.m. peak hour by SCC).

# **Development Phasing**

Given the likely development of the site, individual blocks or groups of buildings would be constructed as existing buildings are removed or undeveloped parking lots are redeveloped. Under a worse-case scenario, a majority of the site could be redeveloped with the exception of the primary Sears Retail building. Under this scenario, the net increase in vehicle trip generation from the overall Shoreline Place development area is estimated to generate up to 160 new p.m. peak hour vehicle trips (in contrast to 99 new p.m. peak hour trips under full redevelopment). This scenario would also fall well below the overall CRA upper threshold of 808 new p.m. peak hour trips, and would be consistent on an interim basis under this worse-case scenario

# Conclusions

Based upon the review of comparative trip generation and land use assumptions, traffic operational and vehicle queueing analysis, and the transportation improvements planned or required, the proposed Shoreline Place project was found to be consistent with the overall assumptions, findings, and conclusions of the Aurora Square CRA and underlying EIS documents, and therefore, no other transportation review is warranted. If you have any questions, comments, or concerns, please do not hesitate to contact me at (206) 361-7333 ext. 101.



# ATTACHMENT A

Conceptual Site Plan



# ATTACHMENT B

Detailed Trip Generation Estimates
Shoreline Sears Redevelopment AM Peak Hour Trip Generation

### Shoreline Place - Preliminary Trip Generation Analysis (Program April 2019 with 1,358 Apts & 72,160K Total Commercial Retail)

A.M. Peak Hour Trip	Generation								
					Trip Rates	5		Trips Generat	ed
			ITE	******	AM Peak Ho	our******	* * * * * *	**AM Peak Hou	ur******
Land Use	Area	Units <sup>1</sup>	LUC <sup>2</sup>	In	Out	Total	In	Out	Total
Subarea A (Proposec	l Residential Core)								
RETAIL	19,625	GFA	820	62%	38%	0.94	11	7	18
Internal Trips	From Residential/Retail						(6)	(2)	(8)
Passby <sup>3</sup>	34%						(1)	(2)	(3)
						Subtotal =	4	3	7
MULTIFAMILY	1,358	UNITS	221	26%	74%	0.36	127	362	489
Internal Trips	Residential-Retail (15%)						(6)	(18)	(24)
						Subtotal =	121	344	464
Calenna D (Dana and	(Detail Come)				Subarea	A Subtotal =	125	34/	4/1
Subarea B (Proposed	(Retail Core)								
RETAIL	39,535	GFA	820	62%	38%	0.94	23	14	37
Internal Trips F	rom Residential Above/Below						(12)	(4)	(16)
Passby	34%					Cubtotal	(3)	(4)	(7)
						Subtotal =	o	0	14
RESTAURANT (2 Sit-Dow	n) 13,000	GFA	931	50%	50%	0.73	5	5	10
						Subtotal =	5	5	10
Deres and the C. Fridada	11				Subarea	B Subtotal =	13	11	24
Kemoval of Existing	Uses								
RETAIL	143,753	GFA	820	62%	38%	0.94	84	51	135
Internal Trips I	From Auto/Tire Center Below						(15)	(8)	(23)
Passby <sup>3</sup>	34%					_	(28)	(17)	(37)
						Subtotal =	41	26	75
WAREHOUSE/STORAGE	72.193	GFA	150	77%	23%	0.17	9	3	12
	· · ·					Subtotal =	9	3	12
OFFICE 4	20,000	CT A	710	<b>94</b> 0/	1.40/	1 17	20	7	0.7
OFFICE	20,000	GFA	710	80%	14%	Subtotal =	20	<u> </u>	23
						Subtotal -	20	2	23
AUTO/TIRE CENTER	50,042	UNITS	942	<b>66</b> %	34%	2.25	74	38	112
Internal Trips	Retail Above (25%)						(15)	(8)	(23)
					Existi	Subtotal =	59 120	30	89 101
	Gross A M Peak Ho	ur Trips (	enerated	from Rede	velonmen	$\frac{1}{4}$ $\frac{1}{2}$ $\frac{1}$	166	788	554
		ur rrips C	ichei aleu		Total Inte	rnal Trips =	(24)	(24)	(50)
				100	s Total Pa	sby Trips =	(21)	(5)	(10)
					Total Evic	ting Trins =	(129)	(62)	(191)
	Net A.M. Peak Ho	ur Trins C	Generated	from Rede	velopmen	t Area =	7	296	303
Notes:			aveu		Net	New Project Trips	142	363	505
<sup>1</sup> GFA is Gross Floor Area.	GLA is Gross Leasable Area.				Pro	piect Pass-by Trips	5	5	10

<sup>1</sup> GFA is Gross Floor Area, GLA is Gross Leasable Area.

<sup>2</sup> Institute of Transportation Engineers, Trip Generation Manual, 10th Edition, 2017 Land Use Codes.

<sup>3</sup> Passby percent existing/proposed retail use based on documented average rate from ITE Trip Generation Handbook.

<sup>4</sup> Trip generation for proposed office use based on ITE, with minimum rate of 1.20 trips per 1,000 sf.

Shoreline Sears Redevelopment PM Peak Hour Trip Generation

### Shoreline Place - Preliminary Trip Generation Analysis (Program April 2019 with 1,358 Apts & 72,160K Total Commercial Retail)

P.M. Peak Ho	Ir Trip Generation									
			-		Trip Rates	5		Trips Generated		
			ITE	******	PM Peak Ho	our**** <u>*</u>	* * * * * *	*PM Peak Hou	r*****	
Land Use	Area	Units '	LUC <sup>2</sup>	ln	Out	l'otal	In	Out	Total	
Subarea A (Pi	oposea kesiaentiai Core)									
RETAIL	19,625	GFA	820	<b>48</b> %	52%	3.81	36	39	75	
Internal Trips	From Residential/Retail	I					(11)	(16)	(27)	
Passby <sup>9</sup>	34%					Subtotal —	<u>(8)</u> 18	(8) 14	(16)	
						Jupitudi =	10	Т	JL	
MULTIFAMILY	1,358	UNITS	221	61%	<b>39</b> %	0.40	331	211	542	
Internal Trips	Residential-Retail (15%)	)					(50)	(32)	(81)	
	rrom Restaurant Below	1				Subtotal =	<u>(4)</u> 277	( <u>)</u> 179	<u>(12)</u> 440	
					Subarea	A Subtotal =	294	186	480	
Subarea B (Pr	oposed Retail Core)								-	
RETAIL	70 575	GFA	820	48%	59%	3.81	72	70	151	
Internal Trips	From Residential/Hotel Above/Below		520	10 /0	JL /0	5.01	(21)	(33)	(54)	
Passby <sup>3</sup>	34%					_	(16)	(17)	(33)	
						Subtotal =	34	29	63	
RESTAURANT	13,000	GFA	931	67%	33%	4.37	38	19	57	
	Residential-Restaurant (20%)	1				_	(8)	(4)	(12)	
					Cubarra	Subtotal =	30	15	45	
Permoval of E	risting Uses				Subarea	b Subtotal =	07	77	100	
	usung Uses									
RETAIL	143,753	GFA	820	48%	52%	3.81	263	285	548	
Internal Trips	From Auto/Tire Center Below	1					(16)	(15)	(31)	
Passby <sup>3</sup>	34%					-	(79)	(97)	(176)	
						Subtotal =	168	173	341	
WAREHOUSE/ST	ORAGE 72,193	GFA	150	27%	73%	0.19	4	10	14	
						Subtotal =	4	10	14	
OFFICE 4	20.000	CE A	710	16%	840/	1 15	4	10	27	
STICE	20,000	UFA	710	10 %	0770	Subtotal =	4	19	23	
AUTO/TIRE CEN	<b>TEK 50,042</b> Potpil Above (2000)	UNITS	942	48%	52%	3.11	75	81	156	
internal mps	Retail ADOVE (20%)					Subtotal =	60	65	125	
					Existi	ng Subtotal =	235	267	502	
	Gross P.M. Peak I	Hour Trips	Generated	1 from Rea	developm	ent Area =	477	348	825	
				Less	Total Inte	rnal Trips =	(85)	(89)	(175)	
				Les	s Total Pas	sby Trips =	(24)	(25)	(49)	
			-	Less	Total Exis	ting Trips =	(235)	(267)	(502)	
	Net P.M. Peak l	Hour Trips	Generated	1 from Red	developm	ent Area =	132	(33)	99	
Notes:					Net	New Project Trips	392	259	650	
GFA is Gross Flo	or Area, GLA is Gross Leasable Area	۱.			Pro	oject Pass-by Trips	24	25	49	

GFA is Gross Floor Area, GLA is Gross Leasable Area.

<sup>2</sup> Institute of Transportation Engineers, Trip Generation Manual, 10th Edition, 2017 Land Use Codes.

<sup>3</sup> Passby percent existing/proposed retail use based on documented average rate from ITE Trip Generation Handbook.

<sup>4</sup> Trip generation for proposed office use based on ITE, with minimum rate of 1.20 trips per 1,000 sf.

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis**

NCHRP 8-51 Internal Trip Capture Estimation Tool									
Project Name:	Shoreline Place		Organization:	TENW					
Project Location:	Aurora Square CRA		Performed By:	MJR					
Scenario Description:	Buildout		Date:	28-Jan-19					
Analysis Year:	2039		Checked By:						
Analysis Period:	PM Peak Hour		Date:						

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)								
	Developm	ent Data ( <i>For In</i>	formation Only)			Estimated Vehicle-Trips		
Land Use	ITE LUCs <sup>1</sup>	Quantity	Units		Total	Entering	Exiting	
Office					0			
Retail	820	59,160			226	108	118	
Restaurant	931	13,000			57	38	19	
Cinema/Entertainment					0			
Residential	221	1,400			558	340	218	
Hotel					0			
All Other Land Uses <sup>2</sup>					0			
Total					841	486	355	

Table 2-A: Mode Split and Vehicle Occupancy Estimates									
Land Lies		Entering Tri	ps			Exiting Trips			
Lanu Ose	Veh. Occ.	% Transit	% Non-Motorized		Veh. Occ.	% Transit	% Non-Motorized		
Office									
Retail	1.90	10%	2%		1.90	10%	2%		
Restaurant	2.40	5%	4%		2.40	5%	4%		
Cinema/Entertainment									
Residential	1.20	10%	7%		1.20	10%	7%		
Hotel									
All Other Land Uses <sup>2</sup>									

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)										
Origin (From)		Destination (To)								
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-A: Internal Person-Trip Origin-Destination Matrix*									
Origin (From)	Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		0	0	0	0	0			
Retail	0		29	0	8	0			
Restaurant	0	6		0	2	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	3	18	0		0			
Hotel	0	0	0	0	0				

Table 5-A: Computations Summary				Table 6-A: Internal Trip Capture Percentages by Land Use			
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Trips	
All Person-Trips	1,236	704	532	Office	N/A	N/A	
Internal Capture Percentage	11%	9%	12%	Retail	4%	17%	
				Restaurant	52%	17%	
External Vehicle-Trips <sup>3</sup>	651	383	268	Cinema/Entertainment	N/A	N/A	
External Transit-Trips <sup>4</sup>	107	62	45	Residential	2%	8%	
External Non-Motorized Trips <sup>4</sup>	56	34	22	Hotel	N/A	N/A	

<sup>1</sup>Land Use Codes (LUCs) from *Trip Generation Informational Report*, published by the Institute of Transportation Engineers.

<sup>2</sup>Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

<sup>3</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

<sup>4</sup>Person-Trips

\*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas Transportation Institute

# ATTACHMENT C

## Aurora Square Planned Action EIS

Land Use and Trip Generation Comparative Analysis

#### Project Trip Generation Summary with Planned Action Trip Threshold Assumptions

			PM Pea	ak	
Alternative	Land Use Assumptions	Enter	Exit	Trips	_
No Action Alternative (All Existing Uses)	WSDOT, Westminster Place & Triangle Property	453	594	1,047	
Planned Action - Alternative 2	Remove & Replace Sears Complexes (~286,000 SF of var retail/office) Construct New 500 Apt units, 125,000 SF retail, and 125,000 SF office	633	812	1,445	
Net Increase with Alternative 2 from Existing		180	218	398	
Planned Action - Alternative 3	Remove & Replace Sears Complexes & Central Market & Mervyns Shopping (~433,000 SF of var retail) Construct New 1,000 Apt units, 250,000 SF retail, and 250,000 SF office	817	1,038	1,855	
Net Increase with Alternative 3 from Existing		364	444	808	
Shoreline Place - February 2019 Program	Remove Sears Complexes (-286,000 SF of var retail/office) Construct 1,400 Apt units, 59,160 SF retail, and 13,000 SF restaurant	585	561	1,146	Shoreline Place is only
Net Increase with Alternative 3 from Existing		132	(33)	99	24.9% of total new trips from Alt 2 and
					12.3% of total new trips from Alt 3.

Source: ITE Trip Generation, 10th Edition & Aurora Square DEIS/FEIS.

Shoreline Place - April 2019 Program with a total of 72,160 commercial retail and 1,358 residential apartment units.

Note: Existing Buildings Removed under the Shoreline Place are Estimated to Generate Approximately 477 PM Peak Hour Trips (37% of total Existing).

# Addendum

Addendum to: City of Shoreline, Aurora Square Planned Action Draft Environmental Impact Statement (December 12, 2014) and Final Environmental Impact Statement (July 24, 2015).

Date Issued: March 8, 2019

### Introduction

This document addends the City of Shoreline, Aurora Square Planned Action Draft Environmental Impact Statement (EIS) and Final Environmental Impact Statement.

The Draft EIS is available at this website:

http://www.shorelinewa.gov/Home/ShowDocument?id=19087

The Final EIS is available at this website:

http://www.shorelinewa.gov/Home/ShowDocument?id=21489

Consistent with the State Environmental Policy Act (SEPA), this addendum has been prepared to correct a reporting error in the trip generation numbers of the Draft EIS and the Final EIS. The trip generation numbers were reported incorrectly in the documents and have been corrected to match the trip generation numbers used in the analysis. There is no change to the analysis of alternatives, significant impacts, or mitigation measures. A notice of this Addendum has been circulated to those receiving the Final EIS.

### Discussion

Table 0-1. PM Peak Hour Trip Generation by Alternative							
	No Action	Phased Growth	Planned Growth				
	Alternative 1	Alternative 2	Alternative 3				
Inbound Trips	553	933	1,313				
Outbound Trips	737	1,159	1,581				
Total Trips	1,289	2,092	2,894				

The Draft EIS identified the PM peak hour trips generated for each of the three alternatives in Chapter 3.3, Table 3-13 on page 3-51.

Table 3-13 (above) shows the trip generation numbers without a reduction for trips occurring within a site that has multiple land uses. The *National Cooperative Highway Research Program (NCHRP) Report 684* methodology estimates the number of trips between land uses within the site

(internal capture), which decreases the total vehicle trips external to the site. The transportation analysis in the EIS used trip generation numbers with a reduction for internal capture to evaluate traffic operations for the alternatives. The data in the Table 3-13 incorrectly reports the trip generation numbers without the internal capture reduction.

### **EIS Corrections**

Based on the above review, make the following changes to the Draft EIS and the Final EIS.

- Table 0-1. PM Peak Hour Trip Generation by Alternative Planned Growth **No Action Phased Growth** Alternative 1 Alternative 2 **Alternative 3** Inbound Trips <del>553</del> <u>453</u> <del>933</del> <u>633</u> <del>1,313</del> <u>817</u> **Outbound Trips** <del>737</del> <u>594</u> <del>1,159</del> <u>812</u> 1,581 <u>1,038</u> **Total Trips** <del>1,289</del> 1,047 <del>2,092</del> <u>1,445</u> 2,894 1,855
- 1. In the Draft EIS, amend Table 3-13 on page 3-51 as corrected below:

Source: KPG <del>2014</del>2019

2. In the Draft EIS, amend Appendix D: Draft Planned Action Ordinance, Section III D (3) (a) Trip Ranges & Thresholds on page 4 as corrected below:

Peak Hour Inbound and Outbound trips	Peak Hour Inbound and Outbound trips during the PM Peak Hour by Alternative						
Phased		Planned					

	No Action Alternative 1	Phased Growth Alternative 2	Alternative 2 Net Trips	Planned Growth Alternative 3	Alternative 3 Net Trips
Inbound Trips	<del>553</del>	<del>933</del> <u>633</u>	<del>380</del> <u>180</u>	<del>1,313</del> <u>817</u>	<del>760</del> <u>364</u>
Outbound Trips	<del>737</del> <u>594</u>	<del>1,159</del> <u>812</u>	4 <del>22</del> <u>218</u>	<del>1,581</del> <u>1,038</u>	<del>8</del> 44 <u>444</u>
Total Trips	<del>1,289</del> <u>1,047</u>	<del>2,092</del> <u>1,445</u>	<del>803</del> <u>398</u>	<del>2,89</del> 4 <u>1,855</u>	<del>1,605</del> <u>808</u>

Source: KPG 20142019

ELINE



3. In the Final EIS, amend Appendix B: Proposed Planned Action Ordinance, Section 3 C (3) Transportation Thresholds as corrected below:

(a) Trip Ranges and Thresholds. The number of new PM Peak hour <del>and daily</del> trips anticipated within the Planned Action Area and reviewed in the FEIS for 2035 are as follows:

	No Action Alternative 1	Phased <u>Growth</u> Alternative 2	Net Trips Alternative 2	PhasedPlannedGrowthAlternative 3	Net Trips Alternative 3
Inbound Trips	<del>553</del> <u>453</u>	<del>933</del> <u>633</u>	<del>380</del> <u>180</u>	<del>1,313</del> <u>817</u>	<del>760</del> <u>364</u>
Outbound Trips	7 <del>37</del> <u>594</u>	<del>1,159</del> <u>812</u>	<u>422 218</u>	<del>1,581<u>1,038</u></del>	<u>844 <u>444</u></u>
Total Trips	<del>1,289</del> <u>1,047</u>	<del>2,092</del> <u>1,445</u>	<del>803</del> <u>398</u>	<del>2,894</del> <u>1,855</u>	<del>1,605</del> <u>808</u>

# ATTACHMENT D

# Comparative Traffic Volume Forecasts at Study Intersections

2018 Turning Movement Counts

2039 PM Peak Hour Turning Movement Forecasts with Shoreline Place

### **Total Entering Volume Analysis**

			Annual
			Growth
CRA EIS	<u>2013</u>	<u>2030</u>	Factor
Greenwood/N 160 Street	970	1,215	1.3%
Dayton Ave/N 160th Street	1,182	1,550	1.4%
Westminster Wy/N 155th St	1,708	2,045	1.1%
Aurora Ave/N 160th Street	3,672	4,505	1.2%
Aurora Ave/N 155th Street	3,946	4,850	1.2%
			1.2%

					TEV 2039
			Annual		with
			Growth	Project	<u>Shoreline</u>
Shoreline Place Consistency Anal	<u>2018</u>	<u>2039</u>	Factor	<b>Traffic</b>	<u>Place</u>
Greenwood/N 160 Street	917	1,184	1.2%	20	1,204
Dayton Ave/N 160th Street	1,387	1,793	1.2%	65	1,858
Westminster Wy/N 155th St	1,665	2,089	1.1%	443	2,532
Dayton Ave/Carlyle Hall Rd	1,084	1,394	1.2%	15	1,409
Sears West Dr/N 160th St	717	956	1.4%	126	1,082
			1.2%		

















#### Shoreline Place

2039 Full Buildout PM Peak Hour Turning Movement Volumes



#### Shoreline Place

2039 Full Buildout PM Peak Hour Turning Movement Volumes



#### Shoreline Place

2039 Full Buildout PM Peak Hour Turning Movement Volumes



# ATTACHMENT E

# Consistency Analysis between CRA EIS and Shoreline Place Traffic Forecasts

				Ne	et New Trip	s Only				
		No Action	<u>2039</u>	Alexan	<u>WSDOT</u>	<b>Shoreline</b>	<u>2039</u>	FEIS 2030	% Difference to	
CRA EIS Study Intersections	<u>2013</u>	<u>(EIS)</u>	No Action <sup>1</sup>	<u>Trips</u>	<u>Trips</u>	Place Trips	with Buildout	Alternative 3	2039 Buildout	2039 LOS
1 Greenwood/N 160 Street	970	1,215	1,274	2	1	3	1,280	1,268	-0.9%	$D^2$
2 Dayton Ave/N 160th Street	1,182	1,550	1,626	4	6	8	1,644	1,839	11.9%	C <sup>2</sup>
3 Aurora Ave/N 160th Street	3,672	4,505	4,725	21	30	25	4,801	4,780	-0.4%	B <sup>2</sup>
4 Aurora Ave/N 155th Street	3,946	4,850	5,087	51	16	39	5,193	5,675	9.3%	$F^3$
5 Westminster Wy/N 155th St	1,708	2,045	2,145	28	0	64	2,237	2,583	15.5%	В
6 Westminster Wy/Dayton Ave	2,116	2,416	2,534	17	35	25	2,611	2,578	-1.3%	B <sup>3</sup>
7 Westminster Wy/Greenwood Ave	2,400	2,880	3,021	17	35	25	3,098	3,037	-2.0%	C <sup>3</sup>
8 Greenwood/N 145 Street	3,204	3,790	3,975	17	35	25	4,052	3,943	-2.7%	E <sup>3</sup>
						A			2 70/	

Average Difference at EIS Study Intersections 3.7%

1 - Factored to 2039 through application of EIS annual average growth rate of 1.2% compounded annually for 4 years given that no growth has occurred in the last 5 years.

It should be noted that factoring 2013 counts (which included both Sears retail complex and retailers within the "triangle" property, i.e., the Alexan site) annually over 20 years factors 2013 site-generated trips by over 30 percent in addition to those trips throughout the vicinity at study intersections.

2 - Source: Shoreline Community College Transportation Technical Report, Transpo Group, October 2018.

3 - Source: Aurora Square DEIS, December 2014.

# ATTACHMENT F

LOS/Vehicle Queuing Summary Sheets

#### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis Lanes, Volumes, Timings 4: Davton Ave & N 160th St 4/5/2019

			~		+		4	*	•	7	I	7
		-	¥	×	-	~	٦	Γ	~	*	ŧ	*
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- ሽ	4			- ₽			4		<u> </u>	î≽	
Volume (vph)	36	206	27	58	186	85	35	698	135	60	301	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	150		0	645		0	250		0	100		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.983			0.953			0.976			0.992	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1831	0	1770	1775	0	1770	1818	0	1770	1848	0
Flt Permitted	0.423			0.497			0.538			0.144		
Satd. Flow (perm)	788	1831	0	926	1775	0	1002	1818	0	268	1848	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11			37			29			8	
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		704			284			543			1479	
Travel Time (s)		13.7			5.5			12.3			33.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	39	224	29	63	202	92	38	759	147	65	327	18
Shared Lane Traffic (%)												
Lane Group Flow (vph)	39	253	0	63	294	0	38	906	0	65	345	0
Enter Blocked Intersection	No	No	No									
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	

Shoreline Place 5:00 pm 2/5/2019 2039 with Project 4-5-19 Program TENW

Synchro 8 Report Page 14

#### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis Lanes, Volumes, Timings 4: Davton Ave & N 160th St 4/5/2019

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	20.0	20.0		20.0	20.0		20.0	20.0		20.0	20.0	
Total Split (s)	20.0	20.0		20.0	20.0		40.0	40.0		40.0	40.0	
Total Split (%)	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%		66.7%	66.7%	
Maximum Green (s)	16.0	16.0		16.0	16.0		36.0	36.0		36.0	36.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		3.5	3.5		3.5	3.5	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Walk Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)	12.4	12.4		12.4	12.4		29.7	29.7		29.7	29.7	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.59	0.59		0.59	0.59	
v/c Ratio	0.20	0.55		0.28	0.63		0.06	0.84		0.41	0.32	
Control Delay	20.0	22.5		20.9	23.0		5.2	17.8		16.0	6.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	20.0	22.5		20.9	23.0		5.2	17.8		16.0	6.3	
LOS	В	С		С	С		A	В		В	Α	
Approach Delay		22.1			22.7			17.2			7.8	
Approach LOS		С			С			В			A	
Intersection Summary												
Area Type:	Other											
Cycle Length: 60	_											
Actuated Cycle Length: 50.	.5											
Natural Cycle: 60												
Control Type: Actuated-Un	coordinated	1										
Maximum v/c Ratio: 0.84	17.0											
Intersection Signal Delay: 1	17.0			lr	ntersection	ILUS: B						
Intersection Capacity Utiliza	ation 77.7%	)		10	JU Level o	or Service	θD					
Analysis Period (min) 15												

#### Splits and Phases: 4: Dayton Ave & N 160th St

	<sub>ø4</sub>
40 s	20 s
ø6	<b>♦</b> Ø8
40 s	20 s

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Queues 4/5/2019

4: Dayton Ave & N 160th St

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	-	-	ŧ		7	I		*
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	39	253	63	294	38	906	65	345
v/c Ratio	0.20	0.55	0.28	0.63	0.06	0.84	0.41	0.32
Control Delay	20.0	22.5	20.9	23.0	5.2	17.8	16.0	6.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.0	22.5	20.9	23.0	5.2	17.8	16.0	6.3
Queue Length 50th (ft)	10	70	17	76	4	187	9	45
Queue Length 95th (ft)	33	135	47	149	15	#472	43	90
Internal Link Dist (ft)		624		204		463		1399
Turn Bay Length (ft)	150		645		250		100	
Base Capacity (vph)	263	618	308	617	734	1341	196	1357
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.41	0.20	0.48	0.05	0.68	0.33	0.25
Intersection Summary								

95th percentile volume exceeds capacity, queue may be longer. #

Queue shown is maximum after two cycles.

### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis

Intersection Capacity Utilization 4: Davton Ave & N 160th St

4: Dayton Ave & N	160th S	St									4	/5/2019
	٦	-	$\mathbf{i}$	∢	←	•	1	Ť	۲	1	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	1		۲	ţ,		۲	ţ,		۲	ţ,	
Volume (vph)	36	206	27	58	186	85	35	698	135	60	301	17
Pedestrians												
Ped Button												
Pedestrian Timing (s)												
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	90	90	90	90	90	90	90	90	90	90	90	90
Volume Combined (vph)	36	233	0	58	271	0	35	833	0	60	318	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.98	0.85	0.95	0.95	0.85	0.95	0.98	0.85	0.95	0.99	0.85
Saturated Flow (vph)	1805	1867	0.00	1805	1811	0.00	1805	1854	0.00	1805	1885	0.00
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Frequency (%)	0.0	0.00	0.0	0.0	0.00	0.0	0.0	0.00	0.0	0.0	0.00	0.0
Protected Ontion Allowed		Ves			Yes			Yes			Yes	
Reference Time (s)	18	11.2	0.0	29	13.5	0.0	17	40.4	0.0	3.0	15.2	0.0
Adi Reference Time (s)	8.0	15.2	0.0	2.5 8.0	17.5	0.0	8.0	40.4	0.0	8.0	19.2	0.0
Permitted Option	0.0	10.2	0.0	0.0	17.0	0.0	0.0		0.0	0.0	10.2	0.0
Adi Saturation A (yph)	160	1867		160	1811		160	185/		160	1885	
Auj Saturation A (vpn)	20.2	11.2		32.5	13.5		10.6	1004		33.7	15.2	
Adi Saturation B (voh	20.2 NA	NIA		52.5 NA	13.5 NA		19.0 NA	40.4 NA		55.7 NA	NA	
Reference Time B (s)												
Poforonco Timo (c)	INA	20.2		INA	32.5		INA	10 A		NA.	22.7	
Adi Deference Time (S)		20.2			36.5			40.4			37.7	
Auj Relefence Time (5)		24.2			30.5			44.4			51.1	
	4.0	44.0		0.0	40 5		47	40.4		2.0	45.0	
Ref Time Combined (s)	1.8	11.2		2.9	13.5		1.7	40.4		3.0	15.2	
Ref Time Seperate (s)	1.8	9.9		2.9	9.2		1.7	33.9		3.0	14.4	
Reference Time (s)	11.2	11.2		13.5	13.5		40.4	40.4		15.2	15.2	
Adj Reference Time (s)	15.2	15.2		17.5	17.5		44.4	44.4		19.2	19.2	
Summary	EB WB		NB SB	Со	mbined							
Protected Option (s)	25.5		52.4									
Permitted Option (s)	36.5		44.4									
Split Option (s)	32.7		63.6									
Minimum (s)	25.5		44.4		69.9							
Right Turns												
Adi Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Internetion Original												
Intersection Summary	tion		77 70/			of Convice						
intersection Capacity Utiliza	lion		11.1%	IC	U Level (	UI SELVICE	;		U			

Reference Times and Phasing Options do not represent an optimized timing plan.

	≯	-	$\mathbf{r}$	1	-	•	1	Ť	1	1	Ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	ĥ		5	ţ,		۲	f,		۲	ĥ	
Volume (veh/h)	36	206	27	58	186	85	35	698	135	60	301	17
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1900	1863	1863	1900	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	39	224	29	63	202	92	38	759	147	65	327	18
Adj No. of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	232	396	51	269	297	135	667	926	179	265	1067	59
Arrive On Green	0.24	0.24	0.24	0.24	0.24	0.24	0.61	0.61	0.61	0.61	0.61	0.61
Sat Flow, veh/h	1081	1617	209	1122	1213	552	1031	1517	294	613	1749	96
Grp Volume(v), veh/h	39	0	253	63	0	294	38	0	906	65	0	345
Grp Sat Flow(s),veh/h/ln	1081	0	1826	1122	0	1765	1031	0	1811	613	0	1846
Q Serve(q s), s	1.9	0.0	6.7	2.9	0.0	8.3	1.0	0.0	21.6	5.1	0.0	4.9
Cycle Q Clear( $q$ c), s	10.2	0.0	6.7	9.6	0.0	8.3	6.0	0.0	21.6	26.7	0.0	4.9
Prop In Lane	1.00		0.11	1.00		0.31	1.00		0.16	1.00		0.05
Lane Grp Cap(c), veh/h	232	0	447	269	0	432	667	0	1105	265	0	1126
V/C Ratio(X)	0.17	0.00	0.57	0.23	0.00	0.68	0.06	0.00	0.82	0.25	0.00	0.31
Avail Cap(c a), veh/h	280	0	529	319	0	511	710	0	1181	291	0	1203
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	23.5	0.0	18.3	22.5	0.0	18.9	6.6	0.0	8.4	18.8	0.0	5.2
Incr Delay (d2), s/veh	0.3	0.0	1.1	0.4	0.0	2.9	0.0	0.0	4.5	0.5	0.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(-26165%),veh/In	0.6	0.0	3.5	0.9	0.0	4.3	0.3	0.0	11.9	0.9	0.0	2.5
LnGrp Delay(d),s/veh	23.8	0.0	19.4	22.9	0.0	21.8	6.6	0.0	12.9	19.3	0.0	5.3
LnGrp LOS	С		В	С		С	А		В	В		A
Approach Vol, veh/h		292			357			944			410	
Approach Delay, s/veh		20.0			22.0			12.6			7.5	
Approach LOS		В			С			В			A	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		37.7		17.5		37.7		17.5				
Change Period (Y+Rc), s		4.0		4.0		4.0		4.0				
Max Green Setting (Gmax), s		36.0		16.0		36.0		16.0				
Max Q Clear Time (g_c+I1), s		23.6		12.2		28.7		11.6				
Green Ext Time (p_c), s		7.6		1.3		5.0		1.5				
Intersection Summary												
HCM 2010 Ctrl Delay			14.3									
HCM 2010 LOS			В									

### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis Lanes, Volumes, Timings 5: Sears West Dr & N 160th St 4/5/2019

	-	$\mathbf{r}$	1	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ef.		7	•	٦	1
Volume (vph)	435	70	58	272	59	111
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.981					0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1827	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1827	0	1770	1863	1770	1583
Link Speed (mph)	35			35	30	
Link Distance (ft)	460			770	202	
Travel Time (s)	9.0			15.0	4.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	473	76	63	296	64	121
Shared Lane Traffic (%)						
Lane Group Flow (vph)	549	0	63	296	64	121
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	ation 49.4%			IC	CU Level	of Service
Analysis Period (min) 15						

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Intersection Capacity Utilization

5: Sears West Dr & N 160th St

4/5/2019

	-	$\mathbf{F}$	∢	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ţ,		3	*	۲	1
Volume (vph)	435	70	58	272	59	111
Pedestrians						
Ped Button						
Pedestrian Timing (s)						
Free Right		No				No
Ideal Flow	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	90	90	90	90	90	90
Volume Combined (vph)	505	0	58	272	59	111
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.98	0.85	0.95	1.00	0.95	0.85
Saturated Flow (vph)	1860	0	1805	1900	1805	1615
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Frequency (%)	0.00			0.00	0.00	
Protected Option Allowed	Yes			Yes	No	
Reference Time (s)	24.4	0.0	2.9	12.9		6.2
Adj Reference Time (s)	28.4	0.0	8.0	16.9		10.2
Permitted Option						
Adj Saturation A (vph)	1860		160	1900	160	
Reference Time A (s)	24.4		32.5	12.9	33.1	
Adj Saturation B (vph	NA		NA	NA	NA	
Reference Time B (s)	NA		NA	NA	NA	
Reference Time (s)	24.4			32.5		
Adj Reference Time (s)	28.4			36.5		
Split Option						
Ref Time Combined (s)	24.4		2.9	12.9	2.9	
Ref Time Seperate (s)	21.0		2.9	12.9	2.9	
Reference Time (s)	24.4		12.9	12.9	2.9	
Adj Reference Time (s)	28.4		16.9	16.9	8.0	
Summary	FB WB		NR	Co	mbined	
Protected Ontion (s)	36.4		NΔ	00		
Permitted Option (s)	36.5		Frr			
Split Option (s)	45.3		8.0			
Minimum (s)	36.4		8.0		44.4	
Dight Turpe	NDD					
	NBR					
Adj Reference Time (S)	10.2					
Cross Inru Ket Ilme (S)	28.4					
Combined (a)	0.0					
	30.0					
Intersection Summary						
Intersection Capacity Utilizati	ion		49.4%	IC	U Level o	of Service

Reference Times and Phasing Options do not represent an optimized timing plan.

### Intersection

Int Delay, s/veh

3.2

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Vol, veh/h	435	70	58	272	59	111
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	0	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	473	76	63	296	64	121

Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	549	0	933	511	
Stage 1	-	-	-	-	511	-	
Stage 2	-	-	-	-	422	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1021	-	295	563	
Stage 1	-	-	-	-	602	-	
Stage 2	-	-	-	-	662	-	
Platoon blocked, %	-	-		-			
Mov Cap-1 Maneuver	-	-	1021	-	277	563	
Mov Cap-2 Maneuver	-	-	-	-	277	-	
Stage 1	-	-	-	-	602	-	
Stage 2	-	-	-	-	621	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0	1.5	16.2	
HCM LOS			С	

Minor Lane/Major Mvmt	NBLn11	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	277	563	-	-	1021	-	
HCM Lane V/C Ratio	0.232	0.214	-	-	0.062	-	
HCM Control Delay (s)	21.9	13.1	-	-	8.8	-	
HCM Lane LOS	С	В	-	-	А	-	
HCM 95th %tile Q(veh)	0.9	0.8	-	-	0.2	-	

### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis Lanes, Volumes, Timings 6: ROCI East Dr & N 160th St 4/5/2019

	-	$\mathbf{F}$	1	+	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ef (				¥		
Volume (vph)	483	14	80	321	33	127	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	1.00	
Frt	0.996				0.893		
Flt Protected				0.990	0.990		
Satd. Flow (prot)	1855	0	0	3504	1647	0	
Flt Permitted				0.990	0.990		
Satd. Flow (perm)	1855	0	0	3504	1647	0	
Link Speed (mph)	35			35	30		
Link Distance (ft)	770			357	181		
Travel Time (s)	15.0			7.0	4.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	525	15	87	349	36	138	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	540	0	0	436	174	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilization	ation 60.5%			IC	CU Level of	of Service B	3
Analysis Period (min) 15							

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Intersection Capacity Utilization

6: ROCI East Dr & N 160th St

4/5/2019

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Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	٦.			<b>≜</b> t₀	M	
Volume (vph)	483	14	80	321	33	127
Pedestrians						
Ped Button						
Pedestrian Timing (s)						
Free Right		No				No
Ideal Flow	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	90	90	90	90	90	90
Volume Combined (vph)	497	0	0	401	160	0
Lane Utilization Factor	1.00	1.00	1.00	0.95	1.00	1.00
Turning Factor (vph)	1 00	0.85	0.95	0.99	0.87	0.85
Saturated Flow (vph)	1892	0.00	0.00	3582	1657	0.00
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Frequency (%)	0.0	0.0	0.0	0.0	0.00	0.0
Protected Ontion Allowed	No			No	No	
Reference Time (s)	NU	0.0		NU	NU	0.0
Adi Reference Time (s)		0.0				0.0
Auj Nelelence Time (5)		0.0				0.0
Adi Seturation A (unb)	1000		0	150	147	
Auj Saluralion A (vpn)	1092		0	159	07.0	
Adi Seturation D (unb	23.0 NA		0.0	40.Z	97.0	
Auj Saluralion B (vpn	INA NA		INA NA	NA NA	NA NA	
Reference Time D (S)	1NA 02.6		INA	1NA 45.0	ΝA	
Adi Deference Time (S)	23.0			40.2		
	27.0			49.Z		
Split Option	00.0		0.0	40.4	07	
Ref Time Combined (s)	23.6		0.0	10.1	8.7	
Ref Time Seperate (s)	23.0		4.0	8.0	1.8	
Reference Time (s)	23.6		10.1	10.1	8.7	
Adj Reference Time (s)	27.6		14.1	14.1	12.7	
Summary	EB WB		NB	Co	mbined	
Protected Option (s)	NA		NA			
Permitted Option (s)	49.2		Err			
Split Option (s)	41.7		12.7			
Minimum (s)	41.7		12.7		54.4	
Pight Turns						
Adj Reference Time (s)						
Cross I nru Ret I me (s)	N					
Combined (a)	)					
Complined (s)						
Intersection Summary						
Intersection Capacity Utiliza	ation		60.5%	IC	U Level o	of Service

Reference Times and Phasing Options do not represent an optimized timing plan.

#### Intersection

Int Delay, s/veh

3.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Vol, veh/h	483	14	80	321	33	127
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	525	15	87	349	36	138

Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	540	0	881	533	
Stage 1	-	-	-	-	533	-	
Stage 2	-	-	-	-	348	-	
Critical Hdwy	-	-	4.12	-	6.63	6.23	
Critical Hdwy Stg 1	-	-	-	-	5.43	-	
Critical Hdwy Stg 2	-	-	-	-	5.83	-	
Follow-up Hdwy	-	-	2.218	-	3.519	3.319	
Pot Cap-1 Maneuver	-	-	1028	-	301	546	
Stage 1	-	-	-	-	587	-	
Stage 2	-	-	-	-	687	-	
Platoon blocked, %	-	-		-			
Mov Cap-1 Maneuver	-	-	1028	-	269	546	
Mov Cap-2 Maneuver	-	-	-	-	269	-	
Stage 1	-	-	-	-	587	-	
Stage 2	-	-	-	-	615	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0	2	18	
HCM LOS			С	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	450	-	-	1028	-	
HCM Lane V/C Ratio	0.386	-	-	0.085	-	
HCM Control Delay (s)	18	-	-	8.8	0.3	
HCM Lane LOS	С	-	-	А	А	
HCM 95th %tile Q(veh)	1.8	-	-	0.3	-	

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Lanes, Volumes, Timings 7: Westminster Way N & Pier 1 Dr

7: Westminster Way	/ N & P	Pier 1 D	Dr								4/5/2019
	4	*	٦	Ť	1	1	Ļ	۶J	<b>`+</b>	$\mathbf{F}$	
Lane Group	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Lane Configurations			ň		1		f,		۲	1	
Volume (vph)	0	0	110	0	28	0	365	74	32	123	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0	0	0		100	100		0	0	0	
Storage Lanes	0	0	1		1	0		0	1	1	
Taper Length (ft)	25		25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.850		0.977			0.850	
Flt Protected			0.950						0.950		
Satd. Flow (prot)	0	0	1593	0	1425	0	1638	0	1593	1425	
Flt Permitted			0.950						0.950		
Satd. Flow (perm)	0	0	1593	0	1425	0	1638	0	1593	1425	
Link Speed (mph)	30			35			35		15		
Link Distance (ft)	179			244			339		204		
Travel Time (s)	4.1			4.8			6.6		9.3		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	120	0	30	0	397	80	35	134	
Shared Lane Traffic (%)											
Lane Group Flow (vph)	0	0	120	0	30	0	477	0	35	134	
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	
Median Width(ft)	0			12			12		12		
Link Offset(ft)	0			0			0		0		
Crosswalk Width(ft)	16			16			16		16		
Two way Left Turn Lane											
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	
Turning Speed (mph)	15	9	15		9	15		9	15	9	
Sign Control	Stop			Free			Free		Stop		
Intersection Summary											
Area Type: C	CBD										
Control Type: Unsignalized											
Intersection Capacity Utilizati	ion 50.9%			IC	CU Level o	of Service	Α				
Analysis Period (min) 15											
Intersection Capacity Utilization 7: Westminster Way N & Pier 1 Dr

4/5/2019

	4	*_	٦	1	1	1	ţ	۶J	4	7	
Movement	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER	
Lane Configurations			1		1		4Î		1	1	
Volume (vph)	0	0	110	0	28	0	365	74	32	123	
Pedestrians											
Ped Button											
Pedestrian Timing (s)											
Free Right		No			No			No		No	
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Refr Cycle Length (s)	90	90	90	90	90	90	90	90	90	90	
Volume Combined (vph)	0	0	110	0	28	0	439	0	32	123	
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Factor (vph)	0.95	0.85	0.95	1.00	0.85	0.95	0.97	0.85	0.95	0.85	
Saturated Flow (vph)	0	0	1625	0	1454	0	1667	0	1625	1454	
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Pedestrian Frequency (%)	0.00			0.00			0.00		0.00		
Protected Option Allowed	No			Yes			Yes		No		
Reference Time (s)		0.0	6.1	0.0	1.7	0.0	23.7	0.0		7.6	
Adj Reference Time (s)		0.0	10.1	0.0	8.0	0.0	27.7	0.0		11.6	
Permitted Option											
Adj Saturation A (vph)	0		144	0		0	1667		144		
Reference Time A (s)	0.0		68.6	0.0		0.0	23.7		19.9		
Adj Saturation B (vph	NA		NA	NA		0	1667		NA		
Reference Time B (s)	NA		NA	NA		0.0	23.7		NA		
Reference Time (s)				68.6			23.7				
Adj Reference Time (s)				72.6			27.7				
Split Option											
Ref Time Combined (s)	0.0		6.1	0.0		0.0	23.7		1.8		
Ref Time Seperate (s)	0.0		6.1	0.0		0.0	19.7		1.8		
Reference Time (s)	0.0		6.1	6.1		23.7	23.7		1.8		
Adj Reference Time (s)	0.0		10.1	10.1		27.7	27.7		8.0		
Summary	WB		NB SB		SE	Con	nbined				
Protected Option (s)	NA		37.8		NA						
Permitted Option (s)	Err		72.6		Err						
Split Option (s)	0.0		37.8		8.0						
Minimum (s)	0.0		37.8		8.0		45.8				
Right Turns	NBR	SER									
Adi Reference Time (s)	8.0	11.6									
Cross Thru Ref Time (s)	0.0	27.7									
Oncoming Left Ref Time (s)	0.0	0.0									
Combined (s)	8.0	39.3									
Intersection Summarv											
Intersection Capacity Utilizatio	n		50.9%	IC	U Level c	f Service			A		

Reference Times and Phasing Options do not represent an optimized timing plan.

#### Intersection

Int Delay, s/veh

1.3

WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	SEL	SER
0	0	110	0	28	0	365	74	32	123
0	0	0	0	0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free	Free	Free	Stop	Stop
-	-	-	-	None	-	-	None	-	None
-	-	0	-	100	-	-	-	0	0
0	-	-	0	-	-	0	-	0	-
0	-	-	0	-	-	0	-	0	-
92	92	92	92	92	92	92	92	92	92
2	2	2	2	2	2	2	2	2	2
0	0	120	0	30	0	397	80	35	134
	WBL 0 0 Stop - 0 0 92 2 0	WBL   WBR     0   0     0   0     Stop   Stop     -   -     -   -     0   -     0   -     92   92     2   2     0   0	WBL   WBR   NBL     0   0   110     0   0   0     Stop   Stop   Free     -   -   -     -   -   0     0   -   -     0   -   -     0   -   -     92   92   92     2   2   2     0   0   120	WBL   WBR   NBL   NBT     0   0   110   0     0   0   0   0   0     Stop   Stop   Free   Free     -   -   -   -     -   -   0   -   -     0   -   -   0   -     0   -   -   0   -     0   -   -   0   -     0   -   -   0   0     92   92   92   92   92     2   2   2   2   2     0   0   120   0	WBL   WBR   NBL   NBT   NBR     0   0   110   0   28     0   0   0   0   0   0     Stop   Stop   Free   Free   Free     -   -   -   None   -   None     -   -   0   -   100   -   100     0   -   -   0   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   -   100   -   100   100   100   100   100   100   100   100   100   100	WBL   WBR   NBL   NBT   NBR   SBL     0   0   110   0   28   0     0   0   0   0   0   0   0     Stop   Stop   Free   Free   Free   Free     -   -   -   None   -     -   -   0   -   100   -     0   -   -   0   -   -   -     0   -   -   0   -	WBL   WBR   NBL   NBT   NBR   SBL   SBT     0   0   110   0   28   0   365     0   0   0   0   0   0   0   0     Stop   Stop   Free   Free	WBL   WBR   NBL   NBR   NBR   SBL   SBT   SBR     0   0   110   0   28   0   365   74     0   0   0   0   0   28   0   365   74     0   0   0   0   0   0   0   0   0     Stop   Stop   Free   None    None	WBL   WBR   NBL   NBR   SBL   SBT   SBR   SEL   SBR   OUNDED   OUNDED

Major/Minor	Major1			Major2			Minor2	
Conflicting Flow All	477	0	0	0	0	0	437	437
Stage 1	-	-	-	-	-	-	437	-
Stage 2	-	-	-	-	-	-	0	-
Critical Hdwy	4.12	-	-	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-
Follow-up Hdwy	2.218	-	-	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1085	-	-	-	-	-	577	620
Stage 1	-	-	-	-	-	-	651	-
Stage 2	-	-	-	-	-	-	-	-
Platoon blocked, %		-	-		-	-		
Mov Cap-1 Maneuver	1085	-	-	-	-	-	513	620
Mov Cap-2 Maneuver	-	-	-	-	-	-	513	-
Stage 1	-	-	-	-	-	-	651	-
Stage 2	-	-	-	-	-	-	-	-

Approach	NB	SB	SE
HCM Control Delay, s	7	0	
HCM LOS			-

Minor Lane/Major Mvmt	NBL	NBT	NBR SE	Ln1 SELn2	SBL	SBT	SBR	
Capacity (veh/h)	1085	-	-	- 620	-	-	-	
HCM Lane V/C Ratio	0.11	-	-	- 0.216	-	-	-	
HCM Control Delay (s)	8.7	-	-	- 12.4	0	-	-	
HCM Lane LOS	А	-	-	- B	А	-	-	
HCM 95th %tile Q(veh)	0.4	-	-	- 0.8	-	-	-	

#### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Lanes, Volumes, Timings 4/5/2019

8: Westminster Way N & N 155th St

Lane Group   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR     Lane Configurations
Lane Configurations   Image of the state of t
Lane Ooting Individual   Image of the product of the produ
Norma (nm)   100
Note   Note <th< td=""></th<>
Storage Lanes   0   0   1   1   1   2   1   0     Taper Length (ft)   25   25   50   25
Taper Length (ft)   25   25   50   25     Lane Util. Factor   1.00   0.95   0.95   1.00   1.00   1.00   1.00   0.88   1.00   1.00   1.00     Ped Bike Factor   0.99   1.00   0.97   1.00   0.98   1.00   0.99     Fit   0.948   0.850   0.850   0.850   0.950   0.952     Fit Protected   0.950   0.950   0.950   0.950   0.950   0.950     Satd. Flow (prot)   0   3034   0   1608   1693   1439   1608   1693   2533   1608   1600   0     Satd. Flow (prot)   0   3034   0   419   1693   1395   440   1693   2476   1199   1600   0     Right Turn on Red   Yes   Yes   Yes   Yes   Yes   Yes   Yes     Satd. Flow (RTOR)   116   78   397   28   1141   349     Link Distance (ft)
Lane Util. Factor   1.00   0.95   0.95   1.00   1.00   1.00   1.00   0.88   1.00   1.00   1.00     Ped Bike Factor   0.99   1.00   0.97   1.00   0.98   1.00   0.99     Frt   0.948   0.850   0.850   0.850   0.952     Flt Protected   0.950   0.950   0.950   0.950     Satd. Flow (prot)   0   3034   0   1608   1693   1439   1608   1693   2533   1608   1600   0     Satd. Flow (prot)   0   3034   0   1608   1693   1395   440   1693   2476   1199   1600   0     Right Turn on Red   Yes   Yes   Yes   Yes   Yes   Yes   Yes   Yes   Satd. Flow (RTOR)   116   78   397   28   118   118   1441   349   118   15   1441   349   118   1441   349   114   1439   116
Ped Bike Factor 0.99 1.00 0.97 1.00 0.98 1.00 0.99   Frt 0.948 0.850 0.850 0.850 0.950 0.952   Flt Protected 0.950 0.950 0.950 0.950 0.950   Satd. Flow (prot) 0 3034 0 1608 1693 1439 1608 1693 2533 1608 1600 0   Satd. Flow (prot) 0 3034 0 1608 1693 1439 1608 1693 2533 1608 1600 0   Satd. Flow (perm) 0 3034 0 419 1693 1395 440 1693 2476 1199 1600 0   Right Turn on Red Yes Jata Jata<
Frt   0.948   0.850   0.850   0.850   0.850   0.950     Fit Protected   0.950   <
Fit Protected 0.950 0.950 0.950 0.950   Satd. Flow (prot) 0 3034 0 1608 1693 1439 1608 1693 2533 1608 1600 0   Flt Permitted 0.248 0.260 0.709 0 3034 0 419 1693 1395 440 1693 2476 1199 1600 0   Right Turn on Red Yes Yes Yes Yes Yes Yes Yes   Satd. Flow (RTOR) 116 78 397 28 1100 28 1441 349   Link Speed (mph) 25 35 35 35 35 1441 349   Travel Time (s) 13.2 10.0 28.1 6.8 11 1 1   Peak Hour Factor 0.87 0.87 0.89 0.89 0.94 <td< td=""></td<>
Satd. Flow (prot) 0 3034 0 1608 1693 1439 1608 1693 2533 1608 1600 0   Flt Permitted 0.248 0.260 0.709 0 3034 0 419 1693 1395 440 1693 2476 1199 1600 0   Right Turn on Red Yes Yes Yes Yes Yes Yes Yes   Satd. Flow (RTOR) 116 78 397 28 28 25 35 35 1608 1693 1411 349 1608 1693 1411 349 17 1441 349 17 110 1
Filt Permitted 0.248 0.260 0.709   Satd. Flow (perm) 0 3034 0 419 1693 1395 440 1693 2476 1199 1600 0   Right Turn on Red Yes Yes Yes Yes Yes Yes Yes   Satd. Flow (RTOR) 116 78 397 28   Link Speed (mph) 25 35 35 35   Link Distance (ft) 483 515 1441 349   Travel Time (s) 13.2 10.0 28.1 6.8   Confl. Peds. (#/hr) 5 3 3 5 1 1 1   Peak Hour Factor 0.87 0.87 0.89 0.89 0.94 0.94 0.94 0.94 0.94
Satd. Flow (perm) 0 3034 0 419 1693 1395 440 1693 2476 1199 1600 0   Right Turn on Red Yes Yes Yes Yes Yes Yes Yes   Satd. Flow (RTOR) 116 78 397 28   Link Speed (mph) 25 35 35 35   Link Distance (ft) 483 515 1441 349   Travel Time (s) 13.2 10.0 28.1 6.8   Confl. Peds. (#/hr) 5 3 3 5 1 1 1   Peak Hour Factor 0.87 0.87 0.89 0.89 0.94 0.94 0.94 0.94 0.94
Right Turn on Red Yes Yes Yes Yes Yes Yes Yes   Satd. Flow (RTOR) 116 78 397 28   Link Speed (mph) 25 35 35 35   Link Distance (ft) 483 515 1441 349   Travel Time (s) 13.2 10.0 28.1 6.8   Confl. Peds. (#/hr) 5 3 3 5 1 1 1   Peak Hour Factor 0.87 0.87 0.89 0.89 0.94 0.94 0.94 0.94 0.94
Satd. Flow (RTOR) 116 78 397 28   Link Speed (mph) 25 35 35 35   Link Distance (ft) 483 515 1441 349   Travel Time (s) 13.2 10.0 28.1 6.8   Confl. Peds. (#/hr) 5 3 3 5 1 1 1   Peak Hour Factor 0.87 0.87 0.89 0.89 0.94 0.94 0.94 0.94 0.94 0.94
Link Speed (mph)   25   35   35   35     Link Distance (ft)   483   515   1441   349     Travel Time (s)   13.2   10.0   28.1   6.8     Confl. Peds. (#/hr)   5   3   3   5   1   1   1   1     Peak Hour Factor   0.87   0.87   0.89   0.89   0.94<
Link Distance (ft)   483   515   1441   349     Travel Time (s)   13.2   10.0   28.1   6.8     Confl. Peds. (#/hr)   5   3   3   5   1   1   1   1     Peak Hour Factor   0.87   0.87   0.89   0.89   0.94 </td
Travel Time (s)   13.2   10.0   28.1   6.8     Confl. Peds. (#/hr)   5   3   3   5   1   1   1   1     Peak Hour Factor   0.87   0.87   0.89   0.89   0.94   0.94   0.94   0.94   0.94   0.94   0.94
Confl. Peds. (#/hr)   5   3   3   5   1   1   1   1     Peak Hour Factor   0.87   0.87   0.89   0.89   0.94 <td< td=""></td<>
Peak Hour Factor   0.87   0.87   0.89   0.89   0.94
Heavy Vehicles (%) 1% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1%
Adi Flow (voh) 0 353 187 256 375 78 163 73 853 81 315 148
Shared Lane Traffic (%)
Lane Group Flow (yph) $0.540$ $0.256$ $375$ $78$ $163$ $73$ $853$ $81$ $463$ $0$
Enter Blocked Intersection No
Lane Alignment Left Left Right Left Right Left Right Left Right Left Right Left Right
Median Width(ft) 12 12 12 12 12
$\frac{12}{12} = \frac{12}{12} = 12$
Crosswalk Width(ft) 16 16 16 16
Two way Left Turn Lane
Headway Eactor 114 114 114 114 114 114 114 114 114 11
Turning Speed (mph) 15 9 15 9 15 9 15 9
Number of Detectors $1$ $1$ $1$ $1$ $1$ $1$ $1$ $1$ $1$ $1$
Detector Template
Leading Detector (ft) 50 50 50 50 50 50 50 50
Trailing Detector (ft) $0$ $0$ $0$ $0$ $0$ $0$ $0$ $0$ $0$
Detector 1 Position(ft) 0 0 0 0 0 0 0 0 0
Detector 1 Size(ft) 50 50 50 50 50 50 50 50 50
Detector 1 Type CI+Ex
Detector 1 Channel
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Turn Type NA pm+pt NA Perm Parm NA pm+py Perm NA
Protected Phases <u>4</u> 3 8 2 3 6
Permitted Phases 8 8 2 2 6
Detector Phase 4 3 8 8 2 2 3 6 6
Switch Phase
$\begin{array}{c} \text{Minimum Initial (s)} \\ 50 \\ 50 \\ 50 \\ 50 \\ 40 \\ 40 \\ 50 \\ 50$

Shoreline Place 5:00 pm 2/5/2019 2039 with Project 4-5-19 Program TENW

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Lanes, Volumes, Timings

8: Westminster Way N & N 155th St

8: Westminster Way	'N & N	l 155th	St								4	/5/2019
	≯	-	$\rightarrow$	4	+	•	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)		28.0		26.0	26.0	26.0	21.0	21.0	26.0	28.0	28.0	
Total Split (s)		29.0		27.0	56.0	56.0	29.0	29.0	27.0	29.0	29.0	
Total Split (%)		34.1%		31.8%	65.9%	65.9%	34.1%	34.1%	31.8%	34.1%	34.1%	
Maximum Green (s)		24.0		22.0	52.0	52.0	24.0	24.0	22.0	24.0	24.0	
Yellow Time (s)		4.0		4.0	3.5	3.5	4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)		1.0		1.0	0.5	0.5	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)		-1.3		-1.3	-1.3	0.0	-1.5	-1.5	0.0	-1.5	-1.5	
Total Lost Time (s)		3.7		3.7	2.7	4.0	3.5	3.5	5.0	3.5	3.5	
Lead/Lag		Lag		Lead					Lead			
Lead-Lag Optimize?												
Vehicle Extension (s)		3.5		3.5	3.0	3.0	2.0	2.0	3.5	2.0	2.0	
Recall Mode		None		None	None	None	Max	Max	None	Max	Max	
Walk Time (s)					7.0	7.0	5.0	5.0		7.0	7.0	
Flash Dont Walk (s)					12.0	12.0	11.0	11.0		14.0	14.0	
Pedestrian Calls (#/hr)					0	0	0	0		0	0	
Act Effct Green (s)		17.7		38.6	39.6	38.3	25.9	25.9	40.1	25.9	25.9	
Actuated g/C Ratio		0.25		0.54	0.55	0.53	0.36	0.36	0.56	0.36	0.36	
v/c Ratio		0.65		0.50	0.40	0.10	1.03	0.12	0.54	0.19	0.78	
Control Delay		22.9		12.3	10.2	2.2	111.1	19.3	5.4	20.6	32.9	
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		22.9		12.3	10.2	2.2	111.1	19.3	5.4	20.6	32.9	
LOS		С		В	В	A	F	В	A	С	С	
Approach Delay		22.9			10.1			22.2			31.1	
Approach LOS		С			В			С			С	
Intersection Summary												
Area Type: C	BD											
Cycle Length: 85												
Actuated Cycle Length: 71.8												
Natural Cycle: 85												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 1.03												
Intersection Signal Delay: 21.	0			Ir	ntersectio	n LOS: C						
Intersection Capacity Utilization	on 83.3%			(	CU Level	of Service	εE					
Analysis Period (min) 15												

#### Splits and Phases: 8: Westminster Way N & N 155th St

<\ ↓ ø2	<b>€</b> ¢ø3	<b>→</b> ø4
29 s	27 s	29 s
ø6	<b>4</b> Ø8	
29 s	56 s	

8: Westminster Way N & N 155th St

	+	4	+	•	•	1	1	1	ţ
Lane Group	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	540	256	375	78	163	73	853	81	463
v/c Ratio	0.65	0.50	0.40	0.10	1.03	0.12	0.54	0.19	0.78
Control Delay	22.9	12.3	10.2	2.2	111.1	19.3	5.4	20.6	32.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	22.9	12.3	10.2	2.2	111.1	19.3	5.4	20.6	32.9
Queue Length 50th (ft)	86	56	86	0	~80	21	45	24	170
Queue Length 95th (ft)	141	93	133	15	#230	60	107	68	#413
Internal Link Dist (ft)	403		435			1361			269
Turn Bay Length (ft)		175			300			100	
Base Capacity (vph)	1160	617	1276	1046	159	610	1768	432	595
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.47	0.41	0.29	0.07	1.03	0.12	0.48	0.19	0.78

#### Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.Queue shown is maximum after two cycles.

4/5/2019

Intersection Capacity Utilization 8: Westminster Way N & N 155th St

4/5/2019

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>↑</b> ĵ≽		٦	<b>↑</b>	1	٦	<b>↑</b>	77	٦	et 🕺	
Volume (vph)	0	307	163	228	334	69	153	69	802	76	296	139
Pedestrians	5		3	3		5	1		1	1		1
Ped Button		Yes			Yes			No			No	
Pedestrian Timing (s)		16.0			19.0			16.0			21.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	2.7	3.7	2.7	3.7	2.7	4.0	3.5	3.5	5.0	3.5	3.5	2.5
Minimum Green (s)	4.0	5.0	4.0	5.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	4.0
Refr Cycle Length (s)	90	90	90	90	90	90	90	90	90	90	90	90
Volume Combined (vph)	0	470	0	228	334	69	153	69	802	76	435	0
Lane Utilization Factor	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.89	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.95	0.85	0.95	1.00	0.85	0.95	1.00	0.85	0.95	0.95	0.85
Saturated Flow (vph)	0	3086	0	1625	1710	1454	1625	1710	2573	1625	1628	0
Ped Intf Time (s)	0.0	0.1	0.3	0.0	0.0	0.5	0.0	0.0	0.1	0.0	0.0	0.1
Pedestrian Frequency (%)		0.07			0.12			1.00			1.00	
Protected Option Allowed		Yes			Yes			Yes			Yes	
Reference Time (s)	0.0	13.8	0.0	12.6	17.6	4.8	8.5	3.6	28.2	4.2	24.1	0.0
Adj Reference Time (s)	0.0	17.8	0.0	16.6	21.7	10.4	12.5	20.0	33.2	9.0	28.1	0.0
Permitted Option												
Adi Saturation A (vph)	0	1543		144	1710		144	1710		144	1628	
Reference Time A (s)	0.0	13.8		142.1	17.6		95.4	3.6		47.4	24.1	
Adj Saturation B (vph	NA	NA		NA	NA		NA	NA		0	1628	
Reference Time B (s)	NA	NA		NA	NA		NA	NA		12.2	24.1	
Reference Time (s)		13.8			142.1			95.4			24.1	
Adj Reference Time (s)		17.8			146.1			99.4			28.1	
Split Option		-			-						-	
Ref Time Combined (s)	0.0	13.8		12.6	17.6		8.5	3.6		4.2	24.1	
Ref Time Seperate (s)	0.0	9.1		12.6	17.6		8.5	3.6		4.2	16.4	
Reference Time (s)	13.8	13.8		17.6	17.6		8.5	8.5		24.1	24.1	
Adi Reference Time (s)	17.8	17.8		21.7	21.7		20.0	20.0		28.1	28.1	
Summary Dratastad Option (a)			10 G		nnbinea							
Protected Option (s)	34.4		40.6									
Permitied Option (s)	140.1		99.4 40.4									
Split Option (S)	39.0		40.1		75.0							
Minimum (S)	34.4		40.6		75.0							
Right Turns	WBR	NBR										
Adj Reference Time (s)	10.4	33.2										
Cross Thru Ref Time (s)	20.0	17.8										
Oncoming Left Ref Time (s)	0.0	9.0										
Combined (s)	30.4	60.0										
Intersection Summary												
Intersection Consoity Litilizet	ion		83 30/			of Sonvice			C			
			00.0/0						E			

Reference Times and Phasing Options do not represent an optimized timing plan.

	≯	-	$\mathbf{r}$	4	-	*	1	1	1	1	Ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>4</b> 12		۲	•	1	۲	•	11	۲	ţ,	
Volume (veh/h)	0	307	163	228	334	69	153	69	802	76	296	139
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	0	1693	1710	1693	1693	1693	1693	1693	1693	1693	1693	1710
Adj Flow Rate, veh/h	0	353	187	256	375	78	163	73	853	81	315	148
Adj No. of Lanes	0	2	0	1	1	1	1	1	2	1	1	0
Peak Hour Factor	0.87	0.87	0.87	0.89	0.89	0.89	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	1	1	1	1	1	1	1	1	1	1	1
Cap, veh/h	0	549	286	467	833	675	239	671	1313	314	432	203
Arrive On Green	0.00	0.27	0.27	0.17	0.49	0.47	0.40	0.40	0.37	0.40	0.40	0.40
Sat Flow, veh/h	0	2130	1064	1612	1693	1431	841	1693	2529	547	1090	512
Grp Volume(v), veh/h	0	276	264	256	375	78	163	73	853	81	0	463
Grp Sat Flow(s),veh/h/ln	0	1608	1501	1612	1693	1431	841	1693	1265	547	0	1602
Q Serve(q s), s	0.0	9.8	10.0	6.5	9.3	2.0	9.7	1.8	15.8	7.1	0.0	15.8
Cycle Q Clear( $q$ c), s	0.0	9.8	10.0	6.5	9.3	2.0	25.5	1.8	15.8	8.8	0.0	15.8
Prop In Lane	0.00		0.71	1.00		1.00	1.00		1.00	1.00		0.32
Lane Grp Cap(c), veh/h	0	431	403	467	833	675	239	671	1313	314	0	635
V/C Ratio(X)	0.00	0.64	0.65	0.55	0.45	0.12	0.68	0.11	0.65	0.26	0.00	0.73
Avail Cap(c a), veh/h	0	632	590	784	1403	1157	239	671	1313	314	0	635
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	0.0	20.8	20.9	12.6	10.7	9.5	28.4	12.3	11.2	15.0	0.0	16.5
Incr Delay (d2), s/veh	0.0	1.9	2.2	1.2	0.4	0.1	14.7	0.3	2.5	2.0	0.0	7.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(-26165%),veh/In	0.0	4.5	4.3	2.9	4.4	0.8	3.8	0.9	5.9	1.2	0.0	8.2
LnGrp Delay(d),s/veh	0.0	22.7	23.1	13.8	11.1	9.6	43.1	12.6	13.7	17.0	0.0	23.7
LnGrp LOS		С	С	В	В	А	D	В	В	В		С
Approach Vol, veh/h		540			709			1089			544	
Approach Delay, s/veh		22.9			11.9			18.1			22.7	
Approach LOS		С			В			В			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2	3	4		6		8				
Phs Duration (G+Y+Rc), s		29.0	14.4	21.0		29.0		35.3				
Change Period (Y+Rc), s		5.0	5.0	5.0		5.0		* 5				
Max Green Setting (Gmax), s		24.0	22.0	24.0		24.0		* 52				
Max Q Clear Time (q c+l1), s		27.5	8.5	12.0		17.8		11.3				
Green Ext Time (p_c), s		0.0	1.0	3.9		3.3		5.5				
Intersection Summary												
HCM 2010 Ctrl Delav			18.3									
HCM 2010 LOS			В									
Notos												

\* HCM 2010 computational engine requires equal clearance times for the phases crossing the barrier.

### Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis Lanes, Volumes, Timings 9: SP NW Dr & N 160th St 4/5/2019

	-	$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	eî.			ę	Y		
Volume (vph)	467	26	30	307	10	88	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.993				0.879		
Flt Protected				0.996	0.995		
Satd. Flow (prot)	1850	0	0	1855	1629	0	
Flt Permitted				0.996	0.995		
Satd. Flow (perm)	1850	0	0	1855	1629	0	
Link Speed (mph)	35			35	30		
Link Distance (ft)	284			460	207		
Travel Time (s)	5.5			9.0	4.7		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	508	28	33	334	11	96	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	536	0	0	367	107	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			12	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilization	ation 49.8%			IC	CU Level of	of Service A	A
Analysis Period (min) 15							

### **Proposed DA - Att. G/Exhibit B - Shoreline Place Transportation Consistency Analysis** Intersection Capacity Utilization

9: SP NW Dr & N 160th St

4/5/2019

	-	$\mathbf{r}$	1	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ţ,			ដ	¥.	
Volume (vph)	467	26	30	307	10	88
Pedestrians						
Ped Button						
Pedestrian Timing (s)						
Free Right		No				No
Ideal Flow	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	90	90	90	90	90	90
Volume Combined (vph)	493	0	0	337	98	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.99	0.85	0.95	1.00	0.86	0.85
Saturated Flow (vph)	1885	0	0	1892	1636	0
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Frequency (%)	0.00			0.00	0.00	
Protected Option Allowed	No			No	No	
Reference Time (s)	-	0.0		-	-	0.0
Adj Reference Time (s)		0.0				0.0
Permitted Option						
Adi Saturation A (vph)	1885		0	966	145	
Reference Time A (s)	23.5		0.0	31.4	60.7	
Adi Saturation B (vph	NA		NA	NA	NA	
Reference Time B (s)	NA		NA	NA	NA	
Reference Time (s)	23.5			31.4		
Adj Reference Time (s)	27.5			35.4		
Split Option						
Ref Time Combined (s)	23.5		0.0	16.0	5.4	
Ref Time Seperate (s)	22.3		1.5	14.5	0.6	
Reference Time (s)	23.5		16.0	16.0	5.4	
Adi Reference Time (s)	27.5		20.0	20.0	9.4	
Cummon/			ND	Ca	mbined	
Summary			INB	0.0	meniam	
Protected Option (s)	NA 05.4		NA			
Permitted Option (s)	35.4		Err			
Split Option (s)	47.6		9.4		44.0	
iviinimum (s)	35.4		9.4		44.8	
Right Turns						
Adj Reference Time (s)						
Cross Thru Ref Time (s)						
Oncoming Left Ref Time (s)						
Combined (s)						
Interpretion Summers						
	tion		40.00/		المنتعرا ال	f Comila
Intersection Capacity Utiliza	tion		49.8%	IC	U Level o	of Service

Reference Times and Phasing Options do not represent an optimized timing plan.

#### Intersection

Int Delay, s/veh

1.8

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Vol, veh/h	467	26	30	307	10	88
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	508	28	33	334	11	96

Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	536	0	921	522	
Stage 1	-	-	-	-	522	-	
Stage 2	-	-	-	-	399	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1032	-	300	555	
Stage 1	-	-	-	-	595	-	
Stage 2	-	-	-	-	678	-	
Platoon blocked, %	-	-		-			
Mov Cap-1 Maneuver	-	-	1032	-	288	555	
Mov Cap-2 Maneuver	-	-	-	-	288	-	
Stage 1	-	-	-	-	595	-	
Stage 2	-	-	-	-	652	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0	0.8	14	
HCM LOS			В	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	507	-	-	1032	-	
HCM Lane V/C Ratio	0.21	-	-	0.032	-	
HCM Control Delay (s)	14	-	-	8.6	0	
HCM Lane LOS	В	-	-	А	А	
HCM 95th %tile Q(veh)	0.8	-	-	0.1	-	

# ATTACHMENT G

Shoreline Community College Traffic Forecasts at Study Intersections 1, 2, & 3

Shoreline Community College | Transportation Technical Report | 2018

Figure 16: Future (2040) With-Project PM Peak Hour Traffic Volumes





HCM 2010 Signalized Intersection Summary 3: Dayton Ave N

Shoreline Community College Future (2040) With-Project Midday Peak Hour Mitigation

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷			\$			\$	
Traffic Volume (veh/h)	106	101	40	15	44	15	20	283	5	5	252	59
Future Volume (veh/h)	106	101	40	15	44	15	20	283	5	5	252	59
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.95	1.00		0.96	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1881	1900	1900	1759	1900	1900	1863	1900	1900	1881	1900
Adj Flow Rate, veh/h	138	131	52	19	57	19	26	368	6	6	327	77
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Percent Heavy Veh, %	1	1	1	8	8	8	2	2	2	1	1	1
Cap, veh/h	168	159	63	28	85	28	90	856	13	64	725	168
Arrive On Green	0.22	0.22	0.22	0.09	0.09	0.09	0.49	0.49	0.49	0.49	0.49	0.49
Sat Flow, veh/h	763	724	288	333	1000	333	53	1729	27	7	1464	340
Grp Volume(v), veh/h	321	0	0	95	0	0	400	0	0	410	0	0
Grp Sat Flow(s),veh/h/ln	1775	0	0	1667	0	0	1809	0	0	1811	0	0
Q Serve(g_s), s	10.3	0.0	0.0	3.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	10.3	0.0	0.0	3.3	0.0	0.0	8.3	0.0	0.0	8.8	0.0	0.0
Prop In Lane	0.43		0.16	0.20		0.20	0.06		0.01	0.01		0.19
Lane Grp Cap(c), veh/h	390	0	0	142	0	0	959	0	0	957	0	0
V/C Ratio(X)	0.82	0.00	0.00	0.67	0.00	0.00	0.42	0.00	0.00	0.43	0.00	0.00
Avail Cap(c_a), veh/h	473	0	0	445	0	0	959	0	0	957	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	22.3	0.0	0.0	26.6	0.0	0.0	9.8	0.0	0.0	9.9	0.0	0.0
Incr Delay (d2), s/veh	9.5	0.0	0.0	5.3	0.0	0.0	1.3	0.0	0.0	1.4	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/In	6.1	0.0	0.0	1.7	0.0	0.0	4.6	0.0	0.0	4.7	0.0	0.0
LnGrp Delay(d),s/veh	31.8	0.0	0.0	32.0	0.0	0.0	11.1	0.0	0.0	11.3	0.0	0.0
LnGrp LOS	С			С			В			В		
Approach Vol, veh/h		321			95			400			410	
Approach Delay, s/veh		31.8			32.0			11.1			11.3	
Approach LOS		С			С			В			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		33.7		17.2		33.7		9.1				
Change Period (Y+Rc), s		4.0		4.0		4.0		4.0				
Max Green Setting (Gmax), s		16.0		16.0		16.0		16.0				
Max Q Clear Time (g_c+I1), s		10.3		12.3		10.8		5.3				
Green Ext Time (p_c), s		2.5		0.6		2.3		0.3				
Intersection Summary												
HCM 2010 Ctrl Delay			18.2									
HCM 2010 LOS			В									

HCM 2010 Signalized Intersection Summary 3: Dayton Ave N

Shoreline Community College Future (2040) With-Project PM Peak Hour Mitigation

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Volume (veh/h)	100	42	51	13	42	19	45	657	19	6	377	29
Future Volume (veh/h)	100	42	51	13	42	19	45	657	19	6	377	29
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.96	1.00		0.97	1.00		0.97	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1881	1900	1900	1792	1900	1900	1881	1900	1900	1881	1900
Adj Flow Rate, veh/h	106	45	54	14	45	20	48	699	20	6	401	31
Adj No. of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	1	1	1	6	6	6	1	1	1	1	1	1
Cap, veh/h	132	56	67	20	64	29	92	1073	30	50	1087	83
Arrive On Green	0.15	0.15	0.15	0.07	0.07	0.07	0.64	0.64	0.64	0.64	0.64	0.64
Sat Flow, veh/h	895	380	456	299	961	427	70	1688	47	6	1710	131
Grp Volume(v), veh/h	205	0	0	79	0	0	767	0	0	438	0	0
Grp Sat Flow(s),veh/h/ln	1731	0	0	1687	0	0	1805	0	0	1848	0	0
Q Serve(g s), s	9.2	0.0	0.0	3.7	0.0	0.0	1.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g c), s	9.2	0.0	0.0	3.7	0.0	0.0	20.4	0.0	0.0	9.0	0.0	0.0
Prop In Lane	0.52		0.26	0.18		0.25	0.06		0.03	0.01		0.07
Lane Grp Cap(c), veh/h	255	0	0	113	0	0	1195	0	0	1220	0	0
V/C Ratio(X)	0.80	0.00	0.00	0.70	0.00	0.00	0.64	0.00	0.00	0.36	0.00	0.00
Avail Cap(c a), veh/h	346	0	0	337	0	0	1195	0	0	1220	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	33.0	0.0	0.0	36.5	0.0	0.0	9.0	0.0	0.0	6.9	0.0	0.0
Incr Delay (d2), s/veh	9.4	0.0	0.0	7.6	0.0	0.0	2.7	0.0	0.0	0.8	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.1	0.0	0.0	2.0	0.0	0.0	11.1	0.0	0.0	4.8	0.0	0.0
LnGrp Delay(d),s/veh	42.4	0.0	0.0	44.1	0.0	0.0	11.7	0.0	0.0	7.8	0.0	0.0
LnGrp LOS	D			D			В			А		
Approach Vol, veh/h		205			79			767			438	
Approach Delay, s/veh		42.4			44.1			11.7			7.8	
Approach LOS		D			D			В			А	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		54.9		15.8		54.9		9.4				
Change Period (Y+Rc), s		4.0		4.0		4.0		4.0				
Max Green Setting (Gmax), s		36.0		16.0		36.0		16.0				
Max Q Clear Time (g c+I1), s		22.4		11.2		11.0		5.7				
Green Ext Time (p_c), s		7.0		0.5		9.8		0.2				
Intersection Summary												
HCM 2010 Ctrl Delay			16.5									
HCM 2010 LOS			В									

HCM Signalized Intersection Capacity Analysis 11: Greenwood Ave N & Innis Arden Way

Shoreline Community College Future (2040) With-Project AM Peak Hour Mitigation

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	۲	1	۲	•	eî.		
Traffic Volume (vph)	28	276	698	128	175	91	
Future Volume (vph)	28	276	698	128	175	91	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		
Frpb, ped/bikes	1.00	0.96	1.00	1.00	0.95		
Flpb, ped/bikes	0.87	1.00	1.00	1.00	1.00		
Frt	1.00	0.85	1.00	1.00	0.95		
Flt Protected	0.95	1.00	0.95	1.00	1.00		
Satd. Flow (prot)	1424	1412	1736	1827	1686		
Flt Permitted	0.95	1.00	0.95	1.00	1.00		
Satd. Flow (perm)	1424	1412	1736	1827	1686		 
Peak-hour factor, PHF	0.75	0.75	0.75	0.75	0.75	0.75	
Adj. Flow (vph)	37	368	931	171	233	121	
RTOR Reduction (vph)	0	310	0	0	19	0	
Lane Group Flow (vph)	37	58	931	171	335	0	
Confl. Peds. (#/hr)	49	7	7			49	
Heavy Vehicles (%)	10%	10%	4%	4%	2%	2%	
Turn Type	Perm	Perm	Split	NA	NA		
Protected Phases			28	28	6		
Permitted Phases	4	4					
Actuated Green, G (s)	15.3	15.3	51.0	51.0	19.0		
Effective Green, g (s)	15.3	15.3	51.0	51.0	19.0		
Actuated g/C Ratio	0.16	0.16	0.52	0.52	0.20		
Clearance Time (s)	4.0	4.0			4.0		
Vehicle Extension (s)	3.0	3.0			3.0		
Lane Grp Cap (vph)	223	222	909	957	329		
v/s Ratio Prot			c0.54	0.09	c0.20		
v/s Ratio Perm	0.03	c0.04					
v/c Ratio	0.17	0.26	1.02	0.18	1.02		
Uniform Delay, d1	35.5	36.0	23.1	12.2	39.1		
Progression Factor	1.00	1.00	0.85	0.98	1.00		
Incremental Delay, d2	0.4	0.6	31.9	0.1	54.7		
Delay (s)	35.8	36.7	51.5	12.0	93.9		
Level of Service	D	D	D	В	F		
Approach Delay (s)	36.6			45.4	93.9		
Approach LOS	D			D	F		
Intersection Summary							
HCM 2000 Control Delay			52.7	Н	CM 2000	Level of Service	
HCM 2000 Volume to Capacity	y ratio		0.93				
Actuated Cycle Length (s)			97.3	S	um of lost	time (s)	
Intersection Capacity Utilizatio	n		70.5%	IC	U Level o	of Service	
Analysis Period (min)			15				

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis 12: Greenwood Ave N & N 160 St Shoreline Community College Future (2040) With-Project AM Peak Hour Mitigation

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			ę	1		\$			4	
Traffic Volume (vph)	26	75	15	15	115	456	20	350	40	154	241	45
Future Volume (vph)	26	75	15	15	115	456	20	350	40	154	241	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0	4.0		4.0			4.0	
Lane Util. Factor		1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes		0.98			1.00	0.99		0.99			0.99	
Flpb, ped/bikes		0.99			0.99	1.00		1.00			1.00	
Frt		0.98			1.00	0.85		0.99			0.99	
Flt Protected		0.99			0.99	1.00		1.00			0.98	
Satd. Flow (prot)		1670			1769	1509		1812			1717	
Flt Permitted		0.90			0.96	1.00		1.00			0.98	
Satd, Flow (perm)		1528			1715	1509		1812			1717	
Peak-hour factor PHF	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adi Flow (vph)	32	91	18	18	140	556	24	427	49	188	294	55
RTOR Reduction (vph)	0	5	0	0	0	46	0	4		0	4	0
Lane Group Flow (vph)	0	136	0	0	158	510	0	496	0	0	533	0
Confl Peds (#/br)	35	100	60	37	100	11	60	+50	37	11	000	35
Confl Bikes (#/hr)	00		00	51		2	00		2			00
Heavy Vehicles (%)	7%	7%	7%	6%	6%	6%	2%	2%	2%	6%	6%	6%
	Dorm		170	Dorm	0	070	<u>Split</u>	2 /0 NIA	270	Split	0	070
Protocted Phases	Feilii	N/A Q		Feilii		1 A A	Spiit	2		JA	16	
Protected Phases	Q	0		Q	0	4 U Q	2	2		40	40	
Actuated Croop C (a)	0	10 0		0	10 0	56.2		20.0			20.2	
Effective Creen g (a)		10.0			10.0	56.2		29.0			20.0	
Actuated a/C Patia		0.10			0.10	0.5		29.0			0.0	
		0.10			0.10	0.50		0.30			0.39	
Vehicle Extension (c)		4.0			4.0			4.0				
		3.0			3.0	025		5.0			075	
Lane Grp Cap (vpn)		282			317	935		540			0/5	
V/s Ratio Prot		0.00			0.00	CU.21		CU.27			CU.31	
V/s Ratio Perm		0.09			0.09	0.12		0.00			0.70	
V/C Ratio		0.48			0.50	0.55		0.92			0.79	
Uniform Delay, d1		35.5			35.6	12.6		33.0			26.0	
Progression Factor		1.00			1.00	1.00		1.00			0.60	
Incremental Delay, d2		1.3			1.2	0.7		20.5			3.6	
Delay (s)		36.8			36.8	13.3		53.5			19.1	
Level of Service		D			D	В		D			B	
Approach Delay (s)		36.8			18.5			53.5			19.1	
Approach LOS		D			В			D			В	
Intersection Summary												
HCM 2000 Control Delay			29.3	H	CM 2000	) Level of S	Service		С			
HCM 2000 Volume to Capacity	y ratio		0.84									
Actuated Cycle Length (s)			97.3	S	um of los	t time (s)			16.0			
Intersection Capacity Utilizatio	n		75.5%	IC	U Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis 11: Greenwood Ave N & Innis Arden Way Shoreline Community College Future (2040) With-Project Midday Peak Hour Mitigation

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	1	٦	•	f,		
Traffic Volume (vph)	71	538	490	157	181	44	
Future Volume (vph)	71	538	490	157	181	44	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		
Frpb, ped/bikes	1.00	0.96	1.00	1.00	0.96		
Flpb, ped/bikes	0.83	1.00	1.00	1.00	1.00		
Frt	1.00	0.85	1.00	1.00	0.97		
Flt Protected	0.95	1.00	0.95	1.00	1.00		
Satd. Flow (prot)	1437	1496	1752	1845	1697		
Flt Permitted	0.95	1.00	0.95	1.00	1.00		
Satd. Flow (perm)	1437	1496	1752	1845	1697		
Peak-hour factor, PHF	0.81	0.81	0.81	0.81	0.81	0.81	
Adj. Flow (vph)	88	664	605	194	223	54	
RTOR Reduction (vph)	0	485	0	0	9	0	
Lane Group Flow (vph)	88	179	605	194	268	0	
Confl. Peds. (#/hr)	62	6	6			62	
Heavy Vehicles (%)	4%	4%	3%	3%	5%	5%	
Turn Type	Perm	Perm	Split	NA	NA		
Protected Phases			28	28	6		
Permitted Phases	4	4					
Actuated Green, G (s)	27.0	27.0	43.0	43.0	18.0		
Effective Green, g (s)	27.0	27.0	43.0	43.0	18.0		
Actuated g/C Ratio	0.27	0.27	0.43	0.43	0.18		
Clearance Time (s)	4.0	4.0			4.0		
Vehicle Extension (s)	3.0	3.0			3.0		
Lane Grp Cap (vph)	387	403	753	793	305		
v/s Ratio Prot			c0.35	0.11	c0.16		
v/s Ratio Perm	0.06	c0.12					
v/c Ratio	0.23	0.44	0.80	0.24	0.88		
Uniform Delay, d1	28.4	30.3	24.8	18.2	39.9		
Progression Factor	1.00	1.00	0.82	0.95	1.00		
Incremental Delay, d2	0.3	0.8	4.3	0.1	28.2		
Delay (s)	28.7	31.1	24.7	17.4	68.1		
Level of Service	С	С	С	В	Е		
Approach Delay (s)	30.8			22.9	68.1		
Approach LOS	С			С	E		
Intersection Summary							
HCM 2000 Control Delay			33.0	Н	CM 2000	Level of Service	
HCM 2000 Volume to Canacity	/ ratio		0.74		2.11 2000		
Actuated Cycle Length (s)	1000		100.0	S	im of lost	time (s)	
Intersection Capacity Utilization	n		58.1%	IC	U Level o	of Service	
Analysis Period (min)			15				

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis 12: Greenwood Ave N & N 160 St Shoreline Community College Future (2040) With-Project Midday Peak Hour Mitigation

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			र्च	1		÷			\$	
Traffic Volume (vph)	5	30	5	15	25	342	5	305	25	438	275	16
Future Volume (vph)	5	30	5	15	25	342	5	305	25	438	275	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0	4.0		4.0			4.0	
Lane Util. Factor		1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes		0.99			1.00	0.98		0.99			1.00	
Flpb, ped/bikes		1.00			0.95	1.00		1.00			1.00	
Frt		0.98			1.00	0.85		0.99			1.00	
Flt Protected		0.99			0.98	1.00		1.00			0.97	
Satd, Flow (prot)		1780			1691	1507		1839			1766	
Flt Permitted		0.98			0.90	1.00		1.00			0.97	
Satd, Flow (perm)		1747			1557	1507		1839			1766	
Peak-hour factor PHF	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Adi Flow (vph)	6	37	6	19	31	422	6	377	31	541	340	20
RTOR Reduction (vph)	0	5	0	0	0	17	0	3	0	0	1	0
Lane Group Flow (vph)	0	44	0	0	50	405	0	411	0	0	900	0
Confl Peds (#/br)	13		24	50	00	30	24	711	50	30	000	13
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	1%	1%	1%	4%	4%	4%
	Porm	NA	070	Porm	N/0		Split	NIA	170	Split		-170
Protected Phases	r enn	8		L CIIII	8	/ f	2 2	2		2001 16	16	
Pormitted Phases	8	0		8	0	40	2	2		40	40	
Actuated Green, G (s)	U	18.0		0	18.0	67.0		21.0			10 0	
Effective Green, g (s)		10.0			10.0	67.0		21.0			49.0	
Actuated a/C Patio		0.18			0.19	07.0		0.21			49.0	
		4.0			0.10	0.07		1.0			0.49	
Vehicle Extension (s)		4.0			4.0			4.0				
		214			0.0	1000		3.0			005	
Lane Grp Cap (vpn)		314			280	1009		300			000	
V/S Ratio Prot		0.00			0.00	CU.19		CU.22			CU.51	
V/s Ratio Perm		0.03			0.03	0.08		4.00			1.04	
V/C Ratio		0.14			0.18	0.38		1.06			1.04	
Uniform Delay, d1		34.5			34.7	1.3		39.5			25.5	
Progression Factor		1.00			1.00	1.00		1.00			0.81	
Incremental Delay, d2		0.2			0.3	0.2		63.9			35.3	
Delay (s)		34.7			35.0	1.5		103.4			55.9	_
Level of Service		U Q			D	A					E	
Approach Delay (s)		34.7			10.4			103.4			55.9	_
Approach LOS		С			В			F			E	
Intersection Summary												
HCM 2000 Control Delay			54.4	Н	CM 2000	) Level of S	Service		D			
HCM 2000 Volume to Capacit	ty ratio		0.96									
Actuated Cycle Length (s)			100.0	S	um of los	st time (s)			16.0			
Intersection Capacity Utilization	on		80.1%	IC	CU Level	of Service			D			
Analysis Period (min)			15									

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis 11: Greenwood Ave N & Innis Arden Way

Shoreline Community College Future (2040) With-Project PM Peak Hour Mitigation

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Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	٦	1	5	•	4Î				
Traffic Volume (vph)	36	324	346	401	163	23			
Future Volume (vph)	36	324	346	401	163	23			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0				
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00				
Frpb, ped/bikes	1.00	0.94	1.00	1.00	0.98				
Flpb, ped/bikes	0.89	1.00	1.00	1.00	1.00				
Frt	1.00	0.85	1.00	1.00	0.98				
Flt Protected	0.95	1.00	0.95	1.00	1.00				
Satd. Flow (prot)	1510	1435	1770	1863	1783				
Flt Permitted	0.95	1.00	0.95	1.00	1.00				
Satd. Flow (perm)	1510	1435	1770	1863	1783				
Peak-hour factor. PHF	0.93	0,93	0,93	0.93	0,93	0.93			
Adi, Flow (vph)	39	348	372	431	175	25			
RTOR Reduction (vph)	0	296	0	0	5	0			
Lane Group Flow (vph)	39	52	372	431	195	0			
Confl Peds (#/hr)	43	13	13	101	100	43			
Confl Bikes (#/hr)	.0	2	10			2			
Heavy Vehicles (%)	6%	6%	2%	2%	3%	3%			
Turn Type	Perm	Perm	Split	NA	NA	0,0			
Protected Phases	i cim	T OIIII	2.8	2.8	6				
Permitted Phases	4	4	20	20	v				
Actuated Green G (s)	14.2	14.2	50.6	50.6	18 1				
Effective Green g (s)	14.2	14.2	50.6	50.6	18.1				
Actuated g/C Ratio	0.15	0.15	0.53	0.53	0.19				
Clearance Time (s)	4.0	4 0	0.00	0.00	4 0				
Vehicle Extension (s)	3.0	3.0			3.0				
Lane Grn Can (vnh)	225	214	943	003	340				
v/s Ratio Prot	225	214	0.21	c0 23	c0 11				
v/s Ratio Perm	0.03	c0 04	0.21	00.20	00.11				
v/c Ratio	0.00	0.24	0 39	0.43	0 57				
I Iniform Delay, d1	35.2	35.6	13.1	13.5	34.9				
Progression Factor	1 00	1 00	0.60	0.59	1 00				
Incremental Delay d2	0.4	0.6	0.00	0.00	69				
Delay (s)	35.6	36.2	8.0	8.1	41.8				
Level of Service	00.0 D	00.2 D	Δ	Δ	-1.0 D				
Annroach Delay (s)	36.1	U	Λ	80	41.8				
Approach LOS	50.1 D			A	-1.0 D				
Intersection Summary									
HCM 2000 Control Delay			20.7	H	CM 2000	Level of Service	)	С	
HCM 2000 Volume to Capaci	ity ratio		0.45						
Actuated Cycle Length (s)			94.9	S	um of lost	time (s)		16.0	
Intersection Capacity Utilizati	on		51.6%	IC	CU Level o	of Service		А	
Analysis Period (min)			15						
c Critical Lane Group									

HCM Signalized Intersection Capacity Analysis 12: Greenwood Ave N & N 160 St Shoreline Community College Future (2040) With-Project PM Peak Hour Mitigation

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			र्स	1		\$			\$	
Traffic Volume (vph)	38	51	6	25	64	197	19	512	38	236	200	32
Future Volume (vph)	38	51	6	25	64	197	19	512	38	236	200	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0	4.0		4.0			4.0	
Lane Util. Factor		1.00			1.00	1.00		1.00			1.00	
Frpb, ped/bikes		1.00			1.00	0.99		1.00			1.00	
Flpb, ped/bikes		1.00			0.99	1.00		1.00			1.00	
Frt		0.99			1.00	0.85		0.99			0.99	
Flt Protected		0.98			0.99	1.00		1.00			0.98	
Satd. Flow (prot)		1777			1767	1526		1853			1777	
Flt Permitted		0.85			0.92	1.00		1.00			0.98	
Satd. Flow (perm)		1548			1641	1526		1853			1777	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	42	57	7	28	71	219	21	569	42	262	222	36
RTOR Reduction (vph)	0	2	0	0	0	33	0	3	0	0	2	0
Lane Group Flow (vph)	0	104	0	0	99	186	0	629	0	0	518	0
Confl. Peds. (#/hr)	8		17	17		8	17		17	8		8
Confl. Bikes (#/hr)												3
Heavy Vehicles (%)	3%	3%	3%	5%	5%	5%	1%	1%	1%	3%	3%	3%
Turn Type	Perm	NA		Perm	NA	pm+ov	Split	NA		Split	NA	
Protected Phases		8			8	46	2	2		46	46	
Permitted Phases	8	-		8	-	8					-	
Actuated Green, G (s)		16.4			16.4	52.7		30.2			36.3	
Effective Green, g (s)		16.4			16.4	52.7		30.2			36.3	
Actuated g/C Ratio		0.17			0.17	0.56		0.32			0.38	
Clearance Time (s)		4.0			4.0			4.0				
Vehicle Extension (s)		3.0			3.0			3.0				
Lane Grp Cap (vph)		267			283	911		589			679	
v/s Ratio Prot						0.08		c0.34			c0.29	
v/s Ratio Perm		c0.07			0.06	0.04						
v/c Ratio		0.39			0.35	0.20		1.07			0.76	
Uniform Delay, d1		34.8			34.6	10.6		32.4			25.5	
Progression Factor		1.00			1.00	1.00		1.00			0.63	
Incremental Delay, d2		0.9			0.8	0.1		56.7			4.0	
Delay (s)		35.7			35.3	10.7		89.0			20.2	
Level of Service		D			D	В		F			С	
Approach Delay (s)		35.7			18.4			89.0			20.2	
Approach LOS		D			В			F			С	
Intersection Summary												
HCM 2000 Control Delay			48.5	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity	ratio		0.84									
Actuated Cycle Length (s)			94.9	S	um of los	t time (s)			16.0			
Intersection Capacity Utilization	۱		79.3%	IC	U Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

# ATTACHMENT H

N 160<sup>th</sup> Street Roadway Cross Section

